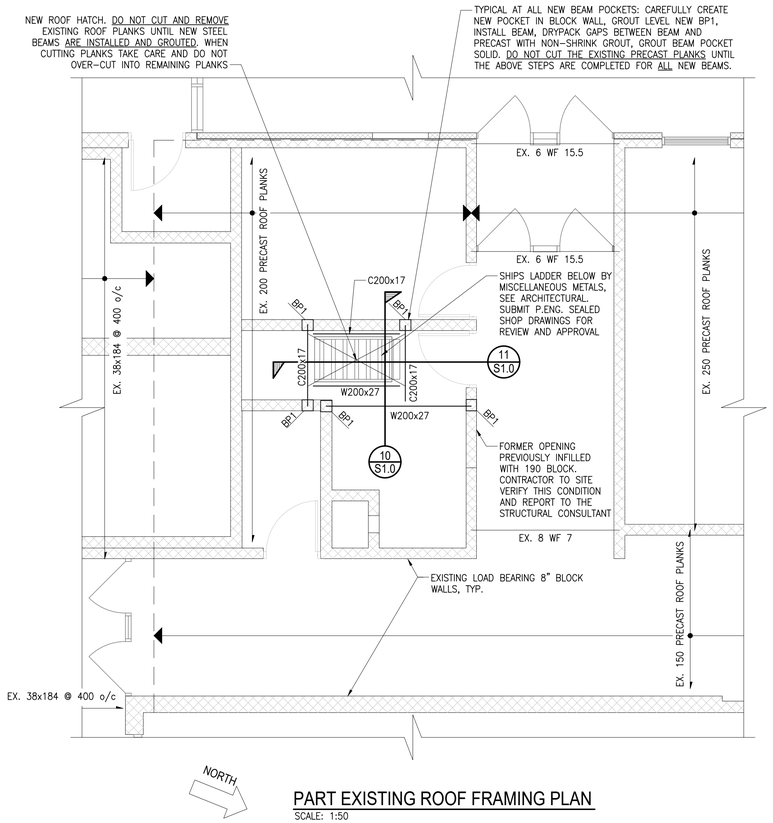


PART EXISTING ROOF FRAMING PLAN

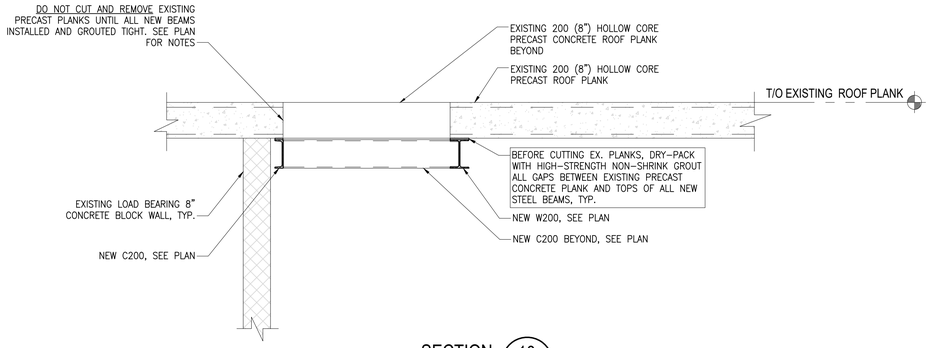
SCALE: 1:50

PART EXISTING ROOF PLAN NOTES

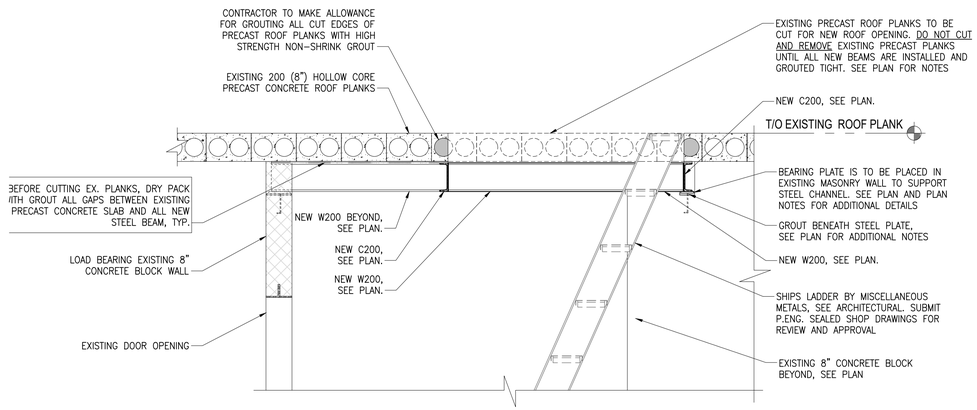
1. CONTRACTOR TO SITE VERIFY ALL ASSUMED CONDITIONS NOTED ON THE CONSULTANT DRAWINGS PRIOR TO CONSTRUCTION AND REPORT DISCREPANCIES TO THE CONSULTANTS. THIS INCLUDES EXISTING CONCRETE BLOCK SIZE, ROOF PLANK BEARING CONDITIONS ON ASSSUMED LOADBEARING BLOCK WALLS. ADDITIONALLY, CONTRACTOR TO REPORT ANY EXISTING DAMAGED, CRACKED, AND SPALLED EXISTING PRECAST ROOF PLANKS IN THE CONSTRUCTION SCOPE.
2. EXISTING ROOF FRAMING CONSISTS OF 8" (203mm) PRECAST HOLLOWCORE PLANKS THAT ARE APPROXIMATELY 16" (406mm) WIDE.
3. REFER TO PLAN FOR DETAILED NOTES REGARDING LOCAL CUTTING AND REMOVAL OF EXISTING PRECAST ROOF PLANKS FOR NEW ROOF OPENING. DO NOT CUT ANY ROOF PLANK UNTIL ALL NEW SUPPORTING STEEL MEMBERS HAVE BEEN INSTALLED AND GROUTED TIGHT.
4. CONTRACTOR IS RESPONSIBLE FOR DESIGN OF ALL TEMPORARY SHORING THAT MAY BE REQUIRED TO PERFORM THE WORK. SUBMIT P.ENG. SEALED SHORING DRAWINGS TO THE CONSULTANTS FOR REVIEW AND APPROVAL.
5. REFER TO ARCHITECTURAL DRAWINGS FOR NEW ROOF HATCH AND ACCESS SHIPS LADDER. COORDINATE NEW ROOF FRAMING TO SUIT OPENING SIZE. MISCELLANEOUS METALS CONTRACTOR TO DESIGN SHIPS LADDER AND ASSOCIATED CONNECTIONS TO NEW ROOF OPENING FRAMING AND EXISTING SLAB ON GRADE. SUBMIT P.ENG. SEALED SHOP DRAWINGS FOR REVIEW AND APPROVAL.
6. GROUT EXISTING BLOCK CORE IMMEDIATELY BELOW ALL NEW BEARING PLATES AND ANCHOR BOLTS.
7. THE STRUCTURAL DRAWINGS MUST BE READ IN CONJUNCTION WITH THE LATEST ARCHITECTURAL DRAWINGS. ALL DIMENSIONS USED IN THE STRUCTURAL DESIGN HAVE BEEN CO-ORDINATED WITH THE ARCHITECTURAL DRAWINGS. THE ARCHITECT OR GENERAL CONTRACTOR MUST INFORM VanBoxmeer & Stranges Ltd OF ANY CHANGES TO ANY DIMENSION THAT AFFECT THE STRUCTURE.

STRUCTURAL STEEL SPECIFICATIONS

1. ALL STRUCTURAL STEEL WORK SHALL BE CARRIED OUT IN ACCORDANCE WITH THE LATEST VERSION OF THE ONTARIO BUILDING CODE, CAN/CSA S16.1, CAN/CSA W59, CAN/CSA W55.3, CAN/CSA W47.1, CAN/CSA S136 AND CAN/CSA G40.21 AND LOCAL BY-LAWS
2. SHOP DRAWINGS:
   - .1 EXAMINE ALL DRAWINGS FORMING A PART OF THIS CONTRACT AND CONFORM TO THE REQUIREMENTS OF ALL SUCH DRAWINGS.
   - .2 ANY FABRICATION EXECUTED BEFORE REVIEW OF SHOP DRAWINGS SHALL BE AT THIS SUB-CONTRACTOR'S RISK. FABRICATION SHALL BE ASSUMED TO BEGIN WHEN MATERIAL IS CUT TO LENGTH, WHETHER THIS BE BY THE FABRICATOR OR AT THE MILL TO THE FABRICATOR'S ORDERS.
   - .3 THE CONSULTANT'S REVIEW OF SHOP DRAWINGS WILL NOT RELIEVE THE SUB-CONTRACTOR FROM THEIR RESPONSIBILITY FOR ENSURING THAT THEIR WORK IS COMPLETE, ACCURATE, AND IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS.
   - .4 SHOP DRAWINGS ARE TO BE SUBMITTED AS FOLLOWS: PDF FORMAT
   - .5 ALL LOADS, FORCES AND REACTIONS SHOWN ON THE DRAWINGS OR NOTED IN THE SPECIFICATIONS ARE SERVICE LOADS (UNFACTORED), UNLESS NOTED OTHERWISE.
   - .6 DESIGN AND DETAILING OF CONNECTIONS, ETC. IN ACCORDANCE WITH CAN/CSA-S16.1. SERVICE LOADS MUST BE FACTORED FOR LIMIT STATES DESIGN.
3. MATERIALS:
   - .1 ROLLED STEEL ANGLES, CHANNELS, PLATES: CURRENTLY PRODUCED IN CANADA IN ACCORDANCE WITH C.S.A. STANDARD G40.21-300W.
   - .2 ROLLED STEEL W-SECTIONS – CSA G40.20/G40.21, GRADE 350W.
   - .3 HIGH STRENGTH BOLTS & WASHERS – IN ACCORDANCE WITH ASTM A325.
   - .4 SHOP PRIMER PAINT – IN ACCORDANCE WITH GSB-1.40.
   - .5 FIELD TOUCH-UP PAINT
     - .1 AS FOR SHOP PAINT FOR PREVIOUSLY SHOP PRIMED MEMBERS
     - .2 GALVAFROID ZINC RICH COATING BY W.R. MEADOWS FOR PREVIOUSLY GALVANIZED MEMBERS.



SECTION 10 / S1.0
SCALE: 1:20



SECTION 11 / S1.0
SCALE: 1:20

DESIGN LOAD DATA

FLOOR/ROOF LOCATION	TYPE OF LOAD			
	DEAD		LIVE	
	CONSTRUCTION	VALUE (kPa)	TYPE	VALUE (kPa)
EX. ROOF	ROOFING ALLOWANCE M&E ALLOWANCE CEILING ALLOWANCE 8" HOLLOW CORE PLANKS	0.55 0.25 0.25 3.54 4.59	SNOW	3.60

OTHER ENVIRONMENTAL LOADS

WIND DATA: q_{50} = 0.36 kPa, Iw = 1.15

SNOW DATA: Ss = 1.6 kPa, Sr = 0.4 kPa, I_s =1.15

SEISMIC DATA: SITE CLASS: D
S(0.2)=0.276 SEISMIC CATEGORY = SC2 (SEE NOTE 6)
S(0.5)=0.26 METHOD OF ANALYSIS:
S(1.0)=0.153 EQUIVALENT STATIC PROCEDURE
S(2.0)=0.073
S(5.0)=0.019
S(10.0)=0.00597
PGA = 0.166
PGV = 0.164

NOTES:

1. ALL LOADS SHOWN ABOVE APPLY TO THE AREAS OF THE BUILDING AS INDICATED, AND ARE BASED ON AND INTERPRETED FROM THE LATEST VERSION OF THE ONTARIO BUILDING CODE. USE THESE LOADS IN DESIGN OF BUILDING COMPONENTS AS REQUIRED.
2. ADDITIONAL SNOW PILE UP IS INCLUDED AS A "DRIFT" DIAGRAM ON ROOF FRAMING PLAN. SNOW DRIFT LOADS ARE IN ADDITION TO THE BASIC SNOW LOADS INDICATED. OVERLAPPING SNOW DRIFT LOADS ARE CUMULATIVE.
3. DEAD LOAD SHOWN IN TABLE IS EXCLUSIVE OF M+E EQUIPMENT. MECHANICAL EQUIPMENT PLAN SIZES AND LOADS ARE SHOWN ON STRUCTURAL PLAN BASED ON THE LATEST INFORMATION AVAILABLE TO THE CONSULTANTS. REPORT ANY DISCREPANCIES BETWEEN CONTRACT DOCUMENTS TO THE CONSULTANTS FOR REVIEW.
4. SEE MECHANICAL DRAWINGS AND ARCHITECTURAL DRAWINGS FOR MECHANICAL EQUIPMENT LOCATIONS AND LOADS.
5. DEAD LOAD SHOWN IN TABLE IS EXCLUSIVE OF MASONRY WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR MASONRY WALL TYPE AND LOCATIONS.
6. THE "SEISMIC CATEGORY" VALUE LISTED ABOVE SHALL BE USED ONLY IN REFERENCE TO THESE STRUCTURAL DOCUMENTS. ALL OTHER CONSULTANTS (NOT STRUCTURAL) MUST VERIFY THIS "SEISMIC CATEGORY" PRIOR TO USING FOR THEIR OWN PURPOSES

STEEL BEARING PLATE SCHEDULE

PLATE MARK	PLATE SIZE	ANCHOR BOLTS	ANCHOR SPACING	COMMENTS
BP1	180x16x180	1-12mmø x 200mm + 50mm HOOK		- GROUT EXISTING BLOCK CORE AROUND NEW A. BOLT

NOTES:

1. STEEL BEARING PLATES TO BE FABRICATED BY STRUCTURAL STEEL CONTRACTOR AND PLACED BY CONCRETE FORMING CONTRACTOR (CONCRETE WALL, BEAM OR COLUMN) OR THE MASONRY CONTRACTOR (CONCRETE BLOCK WALL).
2. LOCATION AND ELEVATION OF BEARING PLATES TO BE COORDINATED WITH STRUCTURAL STEEL SHOP DRAWINGS. BEAM OR JOIST TO BE SUPPORTED IN THE PLATE TO BE CENTERED ALONG THE LENGTH OF THE WALL, UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.
3. DIMENSION OF PLATE NOTED IN PLATE SIZE COLUMN AS 'L' TO BE PLACED ALONG THE LENGTH OF THE WALL, UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.
4. BEAM FLANGE OR JOIST SHOE TO BE FIELD WELDED DOWN TO TOP OF BEARING PLATE USING A MINIMUM 6mm FILLET WELD EACH SIDE OF BEAM/JOIST SHOE FOR THE ENTIRE LENGTH OF BEARING.
5. FOR BEARING PLATES PLACED ON CONCRETE BLOCK WALLS – GROUT SOLID THE CORES OF THE BLOCKS CONTAINING THE ANCHOR BOLTS OF THE PLATES CONTINUOUS TO THE TOP OF THE FOUNDATION WALL. GROUT SOLID THE BEAM/JOIST POCKETS AFTER FIELD WELDING.
6. ANCHOR BOLTS SHOWN TO BE WELDED TO THE UNDERSIDE OF THE BEARING PLATE FOR TENSILE CAPACITY OF THE BOLT.

NOTE:
THE CONTRACTOR SHALL CHECK ALL DIMENSIONS AGAINST ARCHITECTURAL DRAWINGS AND MUST REPORT ANY DISCREPANCIES TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.
DO NOT SCALE DRAWING.

NOTES

No.	REMARKS	DATE
2	ISSUED FOR PERMIT/TENDER	2026/03/09
1	ISSUED FOR COORDINATION	2026/02/06

REVISIONS

LICENSED PROFESSIONAL ENGINEER
P.S. RIZZO
100079724
2026-03-09
PROVINCE OF ONTARIO

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PROJECT
WCDSB ST. JOSEPH'S CES NEW ROOF HATCH

CAMBRIDGE ONTARIO

DRAWING
ROOF PLAN, SCHEDULES & SECTIONS

STATUS: STATUS		PLDT: PLOTDATE
DRAWN BY:	EA	SHEET No.
CHECKED BY:	PR	**S1.0**
SCALE:	SCALE	
DATE:	02/10/2026	
PROJECT No.	26101	