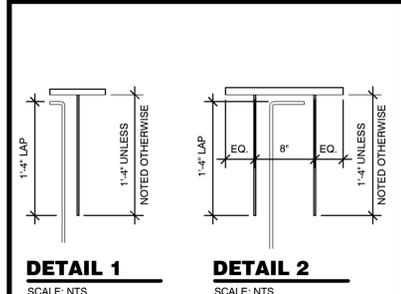
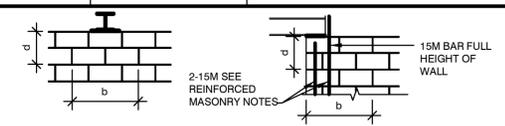


BEARING PLATE SCHEDULE					
MARK	SIZE	ANCHOR SIZE	HOLD DOWN BAR	REMARKS	MINIMUM GROUTING bxd
BP1	200x150x16	16mm B6 THREADED ROD EPOXIED IN TOP OF MASONRY BLOCK GROUTED SOLID. 100mm EMBEDMENT MINIMUM.		BASE PLATE ANCHORED TO TOP OF EXISTING BLOCK WALL.	

- NOTES:**
- ALL BEAMS AND JOISTS ARE TO BE PROVIDED WITH A BEARING PLATE. LINTELS ARE TO BE PROVIDED WITH A BEARING PLATE WHEN INDICATED ON THE DRAWINGS.
 - THE FIRST AND LAST BEARING PLATES ARE SHOWN AS THUS "BP#" ON PLAN AND ALL BEAMS AND JOISTS BETWEEN ARE TO BE PROVIDED THE SAME BEARING PLATE.
 - FILL BLOCK CORES SOLID BELOW BEARING PLATE 1'-4" LONG BY 2 COURSES DEEP UNLESS NOTED OTHERWISE.
 - CORES AT ALL HOLD DOWN BARS TO BE FILLED SOLID WITH 25MPa GROUT.
 - AT HOLD DOWN BAR SPLICE LOCATIONS, LAP BARS 2'-0" MIN.
 - AT ALL HOLD DOWN BARS, PROVIDE STANDARD HOOK AT U/S OF BEARING PLATES.



GENERAL

- GENERAL CONTRACTOR TO SITE VERIFY ALL CONDITIONS AND OR DIMENSIONS SHOWN OR IMPLIED ON THE STRUCTURAL DRAWINGS.
- GENERAL CONTRACTOR TO COORDINATE ALL STRUCTURAL DOCUMENTS AND WORK WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND SITE SERVICING DOCUMENTS AND WORK.
- REPORT ANY DISCREPANCIES AND OR CONFLICTS IN DIMENSIONS OR DETAILS TO THE ARCHITECT FOR CLARIFICATION PRIOR TO COMMENCING THE WORK IN QUESTION.
- PROVIDE ALL TEMPORARY NEEDLING, SHORING AND BRACING AS REQUIRED TO SAFELY COMPLETE THE WORK SHOWN ON THE STRUCTURAL DRAWINGS. SUBMIT, UPON REQUEST OF THE STRUCTURAL ENGINEER, DRAWINGS DETAILING THE TEMPORARY WORKS, SEALED SIGNED AND DATED BY A LICENSED PROFESSIONAL ENGINEER.
- PROVIDE ALL FALSE WORK AND RESHORING REQUIRED TO CONSTRUCT CONCRETE WALLS, COLUMNS AND BEAMS SHOWN ON THE STRUCTURAL DRAWINGS.
- ALL OPENINGS IN MASONRY WALLS REQUIRE A LINTEL. COORDINATE LOCATIONS AND CLEAR MASONRY OPENING WIDTHS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS AND SELECT THE APPROPRIATE LINTEL FROM "LINTEL SCHEDULE-GENERAL PURPOSE LINTELS" AS SHOWN IN THE SCHEDULE ON DRAWING S1.01, USING WALL CONSTRUCTION AND MAXIMUM CLEAR OPENING WIDTH AS THE CRITERIA.

CONCRETE AND CONCRETE REINFORCING

- ALL CONCRETE WORK INCLUDING MATERIALS, MIXING, PLACING, FINISHING, CURING, COLD WEATHER PROTECTION, HOT WEATHER PROTECTION, FORMWORK AND RESHORING IN ACCORDANCE WITH A23.1 AND A23.3 CURRENT UNLESS NOTED OTHERWISE HERE.
- ALL CONCRETE REINFORCING INCLUDING MATERIALS, FABRICATION, DETAILING, LAP SPLICES, PLACEMENT, FIXING AND COVER IN ACCORDANCE WITH A23.1 AND A23.3 CURRENT UNLESS NOTED OTHERWISE HERE.
- WELDED WIRE MESH TO BE LAPPED ONE (1) FULL MESH PLUS 50mm. PROVIDE CONCRETE BRICK BOLSTERS FOR WELDED WIRE MESH AT 1000mm ON CENTRE EACH WAY.
- CURE ALL SLABS ON GRADE WITH A PRE-APPROVED CURING COMPOUND COMPATIBLE WITH THE PROPOSED FLOORING ADHESIVE. MAINTAIN AIR TEMPERATURE AT OR ABOVE 10°C FOR SEVEN (7) DAYS. CONCRETE PROPERTIES:

LOCATION	SPEC. 28 DAY COMPRESSIVE STRENGTH	SLUMP	AIR CONTENT	EXPOSURE CLASS
SUSPENDED SLABS (EXTERIOR)	35MPa (56 DAYS)	100 MAX. (w/c -0.40)	5-8%	C-1
WALLS FOUNDATION (EXTERIOR)	35MPa (56 days)	100 MAX (WC ratio 0.55)	4-7%	F-2
INTERIOR SLAB ON GRADE AND DECK AND FOOTINGS	25MPa	100 MAX		N
HOUSEKEEPING PAD	20MPa	100 MAX		N
SLAB ON DECK	25MPa MAX 10MM AGGREGATE	100 MAX		N

- REINFORCING STEEL TO BE GRADE 400, WITH A MINIMUM SPECIFIED YIELD STRENGTH OF 400 MPA.
- SUBMIT REINFORCING STEEL SHOP DRAWING TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

STRUCTURAL STEEL

- SUBMIT FOR CONSULTANT'S REVIEW ERECTION DIAGRAMS AND FABRICATION DETAILS IN ACCORDANCE WITH THE GENERAL NOTES.
- CONSULTANT'S REVIEW OF SHOP DRAWING DOES NOT RELEASE THE CONTRACTOR OF HIS RESPONSIBILITY FOR THE COMPLETENESS OF THE WORK NOR COORDINATION WITH OTHER TRADES.
- FABRICATION AND ERECTION OF STEEL SHALL BE IN ACCORDANCE WITH CSA S16 (CURRENT).

SHAPE	STANDARD	GRADE	MIN. YIELD STRENGTH
W BEAMS	CSA G40.21	350W	50 Ksi
HOLLOW STRUCTURAL CLASS H UNLESS NOTED AS CLASS C	CSA G40.21	350W	50Ksi
PLATES, CHANNELS AND ANGLES	CSA G40.21	300W	50Ksi

- STRUCTURAL STEEL TO CONFORM TO THE FOLLOWING TABLE UNLESS NOTED OTHERWISE.
- ALL WELDING SHALL BE DONE BY AN ORGANIZATION FULLY APPROVED BY THE CANADIAN WELDING BUREAU UNDER CSA-W47.1 (CURRENT) IN DIVISION 1 OR 2 AT THE TIME OF TENDERING. WELDERS TO BE CWB CERTIFIED TO THE COMPANIES STANDARDS. WELDING AND WELDING MATERIALS SHALL CONFORM TO CSA-W59 (CURRENT).
- THE FABRICATOR SHALL NOTE THE SIZE AND TYPE OF BOLTS AND WELDS USED IN STRUCTURAL CONNECTIONS ON THE SHOP DRAWINGS.
- ALL STRUCTURAL STEEL SHALL BE SUFFICIENTLY STRAIGHT THAT VARIATIONS CANNOT BE DETERMINED WITH THE UNAIDED EYE. ALL STRUCTURAL STEEL SHALL BE THOROUGHLY CLEANED OF ALL LOOSE MILL SCALE, DIRT, OIL, OR OTHER FOREIGN MATTER BEFORE SHOP PAINTING. SHOP PAINT SHALL CONFORM TO CAN/C695 1.40-M89 OR CISC/CPMA STANDARD 2-75.
- ALL STRUCTURAL STEEL EXPOSED TO THE ELEMENTS TO BE HOT DIP GALVANIZED UNLESS NOTED OTHERWISE.
- WHERE HOT DIP GALVANIZING (HDG) IS SPECIFIED IT SHALL BE IN ACCORDANCE WITH CAN/CSA-G164-M92 (MINIMUM ZINC COATING 600 GSM).
- STEEL LINTELS SHALL HAVE A MINIMUM BEARING LENGTH OF 200.

REINFORCED MASONRY NOTES

- VERTICAL MASONRY REINFORCING STEEL TO BE $F_y = 400$ MPA.
- LAP ALL VERTICAL REINFORCING 600MM UNLESS NOTED OTHERWISE.
- MASONRY UNITS FOR LOADBEARING WALLS:
 - UNLESS NOTED OTHERWISE IN WALL SCHEDULE OR ELSEWHERE IN THE STRUCTURAL DOCUMENTS TO BE 15MPa.
- FOR LOAD BEARING WALLS MORTAR TYPE 'S'
- GROUT TO BE 25MPa WITH 10MM MAX AGGREGATE SIZE AND 150mm ± 25mm SLUMP. GROUTING TO CONFORM TO REQUIREMENTS OF CSA STANDARD CAN3-A371-94. ALL GROUTING TO BE 'LOW LIFT'. MAXIMUM LIFT 1000mm.
- UNLESS SPECIFICALLY DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS VERTICAL REINFORCING TO BE LOCATED IN CENTRE OF BLOCK.
- IN ADDITION TO THE VERTICAL REINFORCING SPECIFIED ON DRAWINGS PROVIDE A MINIMUM OF 2-15M EACH SIDE OF:
 - ALL DOOR OPENINGS
 - ALL WINDOW OPENINGS
 - AT ENDS OF ALL WALLS
 - EACH SIDE OF EVERY MASONRY WALL CONTROL JOINT
- ALL MASONRY WALLS ARE TO HAVE LADDER TYPE CONTINUOUS HORIZONTAL WALL REINFORCING AT 400 ON CENTRE. MINIMUM WIRE DIAMETER TO BE 4.76mm. PROVIDE PREFABRICATED CORNER AND TEE HORIZONTAL REINFORCING ALL CORNERS.

MINIMUM TESTING REQUIREMENTS FOR MASONRY

MASONRY UNITS, MORTAR AND GROUT SHALL BE TESTED IN ACCORDANCE WITH THE ONTARIO BUILDING CODE AND REQUIREMENTS OF CSA S304.1-04, BUT NOT LESS THAN THE FOLLOWING:

- UNITS:** ONE SET OF FIVE MASONRY UNITS DELIVERED TO SITE FOR EACH 250sqm OF WALL OF EACH TYPE OF MASONRY UNIT, IN ACCORDANCE WITH CSA S304, TESTED AND REPORTED PRIOR TO UNITS BEING USED ON SITE. PROVIDED CURRENT DATA IS AVAILABLE FROM THE MANUFACTURE, TESTING OF 15 MPa UNITS IS AVOIDED. CURRENT IN THIS CURRENT DATA MEANS COMPRESSIVE STRENGTH TESTS FOR UNITS IN ACCORDANCE WITH THE REQUIREMENTS OF CSA S304.1-04, PERFORMED WITHIN THE THREE MONTHS PRECEDING THE START OF MASONRY FOR THIS PROJECT.
- MORTAR:** ONE SET OF MORTAR CUBES PER 500 SQUARE METERS OF CONSTRUCTION, OR PART THERE OF IN ACCORDANCE WITH THE TEST PROCEDURES AS PUT FORTH IN THE APPROPRIATE CSA STANDARDS.
- GROUT:** ONE SET OF CUBES AND ONE SLUMP TEST TAKEN AT THE SAME TIME AND FREQUENCY AS MORTAR TEST DESCRIBED ABOVE.

WIND LOADING ON WALL SYSTEMS

IN ADDITION TO THE SEISMIC PROVISIONS OF THE ONTARIO BUILDING CODE THE FOLLOWING TABLES PROVIDE THE MINIMUM SPECIFIED WIND LOADS TO BE USED IN THE DESIGN OF THE NOTED WALL SYSTEMS. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THE CONTRACTORS ENGINEER TO ENSURE THE WALL SYSTEM HAS BEEN DESIGNED TO THE ONTARIO BUILDING CODE BUT NOT LESS THAN THE VALUES GIVEN HERE.

STEEL STUDS, CURTAIN WALL

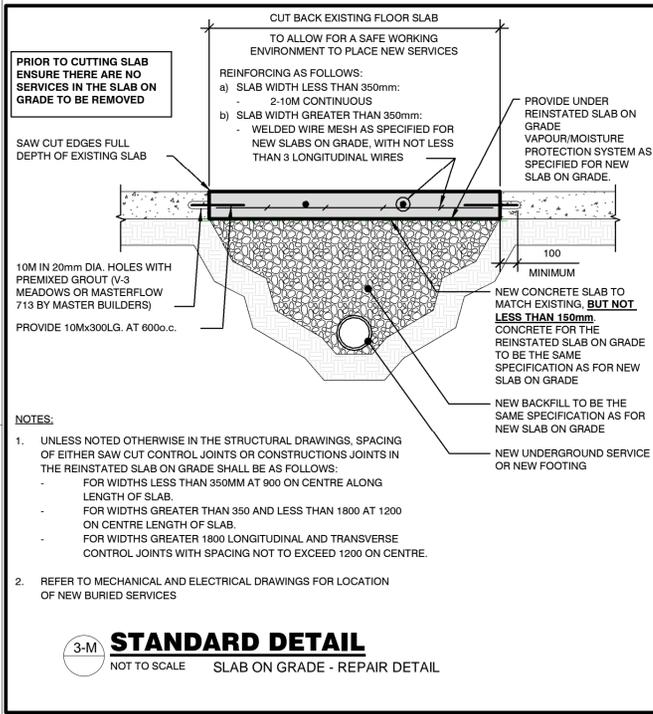
LEVEL	SPECIFIED PRESSURE OR SUCTION LOADS	
	ULS (kPa)	SLS (kPa)
MAIN ROOF TO GYM ROOF	1.07	0.70

$C_p C_g$ DENOTES THE PRODUCT OF THE EXTERNAL PRESSURE COEFFICIENT TIMES THE GUST FACTOR AS NOTED IN FIGURE 4.1.7.6-B OF THE NBC 2015 STRUCTURAL.

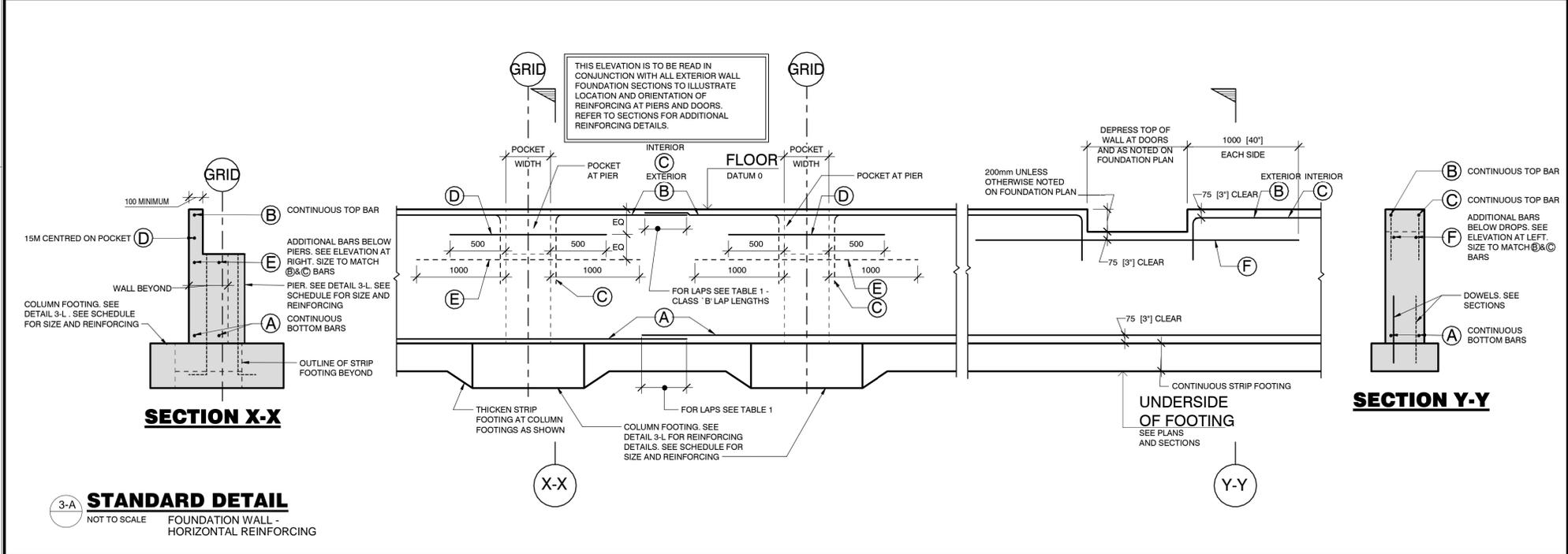
$C_p C_g$ VALUES USED IN THE TABLE ABOVE ARE BASED ON A TRIBUTARY AREA OF 20m² IN FIGURE 4.1.7.6-B.

ULS LOAD FACTOR OF 1.4 NOT INCLUDED IN SPECIFIED ULS LOADS NOTED IN TABLE ABOVE.

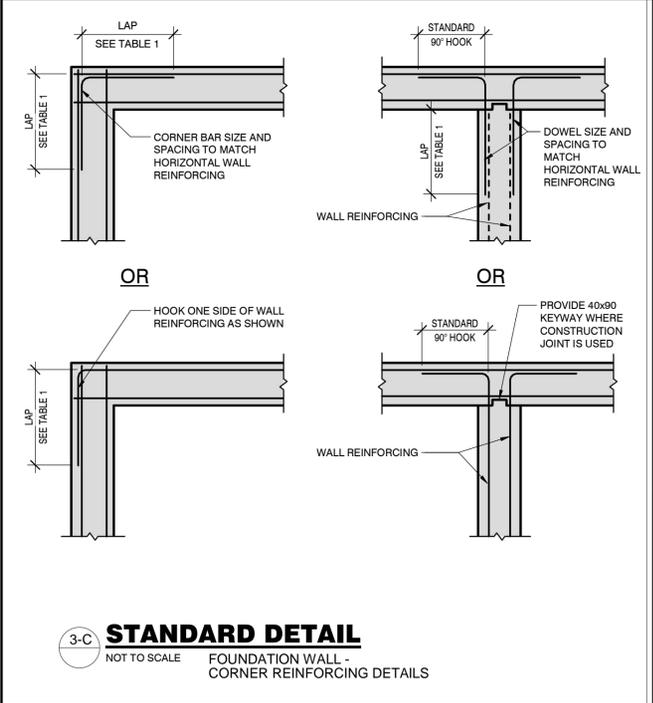
- $C_{pi} = -0.45$ to $+0.30$
- $C_{gi} = 2.0$



3-M STANDARD DETAIL
NOT TO SCALE SLAB ON GRADE - REPAIR DETAIL



3-A STANDARD DETAIL
NOT TO SCALE FOUNDATION WALL - HORIZONTAL REINFORCING



3-C STANDARD DETAIL
NOT TO SCALE FOUNDATION WALL - CORNER REINFORCING DETAILS



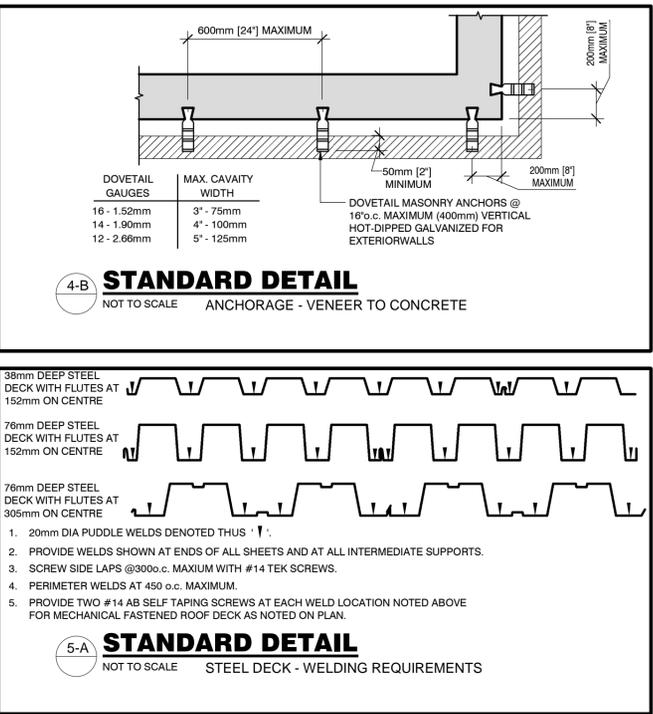
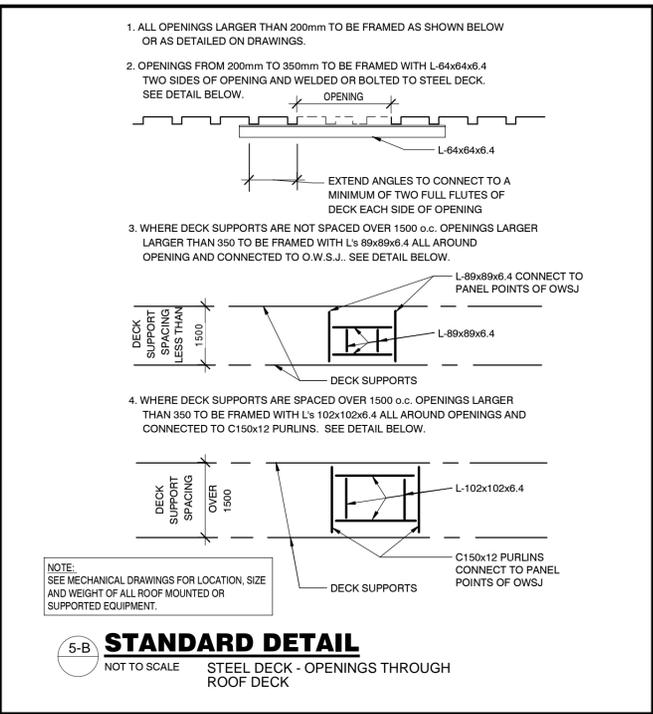
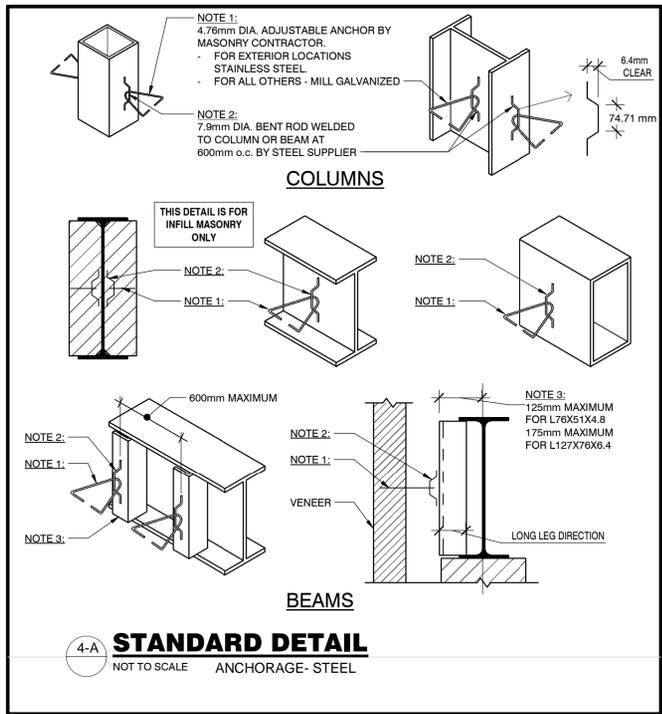
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200 WATER ST. N. CAMBRIDGE, ON
Project No.: 64947_001
Drawn By: JAS
Pur Date: 02/06/26

Galt Collegiate Institute
EXTERIOR / INTERIOR UPGRADES TO TASSIE HALL
STRUCTURAL DATA, TABLES AND DETAILS

S1.01



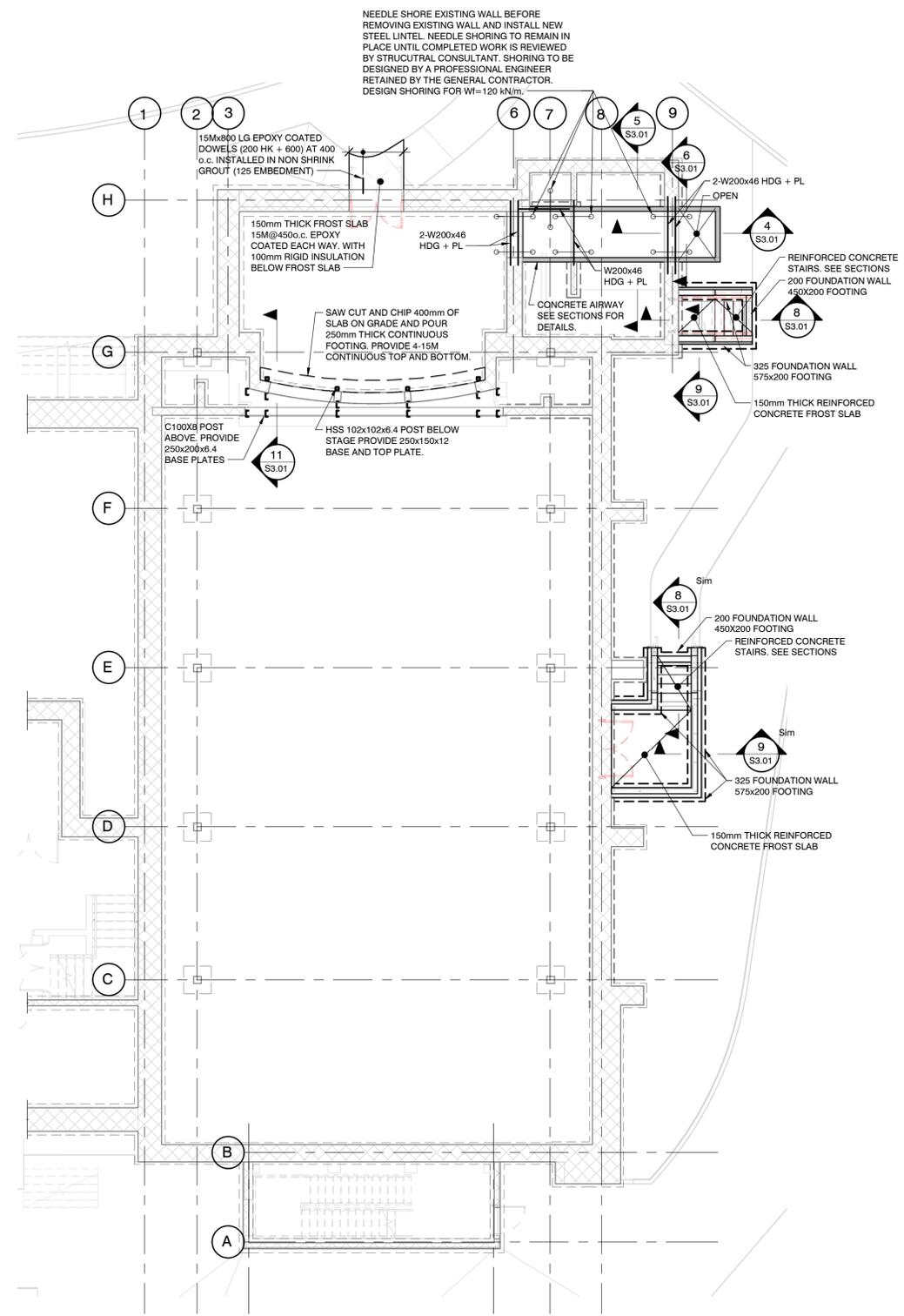
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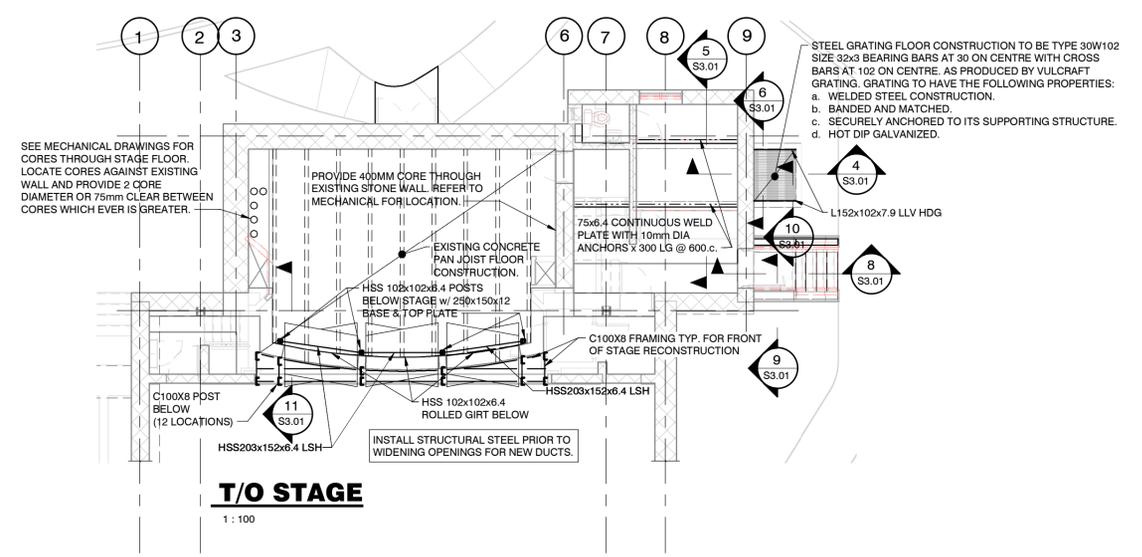
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EXTERIOR / INTERIOR UPGRADES TO TASSIE HALL

DIVISION 4 & 5 STANDARD DETAILS

Project No.: 64947_001
Drawn By: JAS
Plot Date: 02/16/26



FOUNDATION PLAN
 1 : 100



T/O STAGE
 1 : 100

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 EXTERIOR / INTERIOR UPGRADES TO TASSIE HALL
FOUNDATION PLAN & STAGE FRAMING
 Project No.: 64947_001
 Drawn By: JAS
 Plot Date: 02/06/26



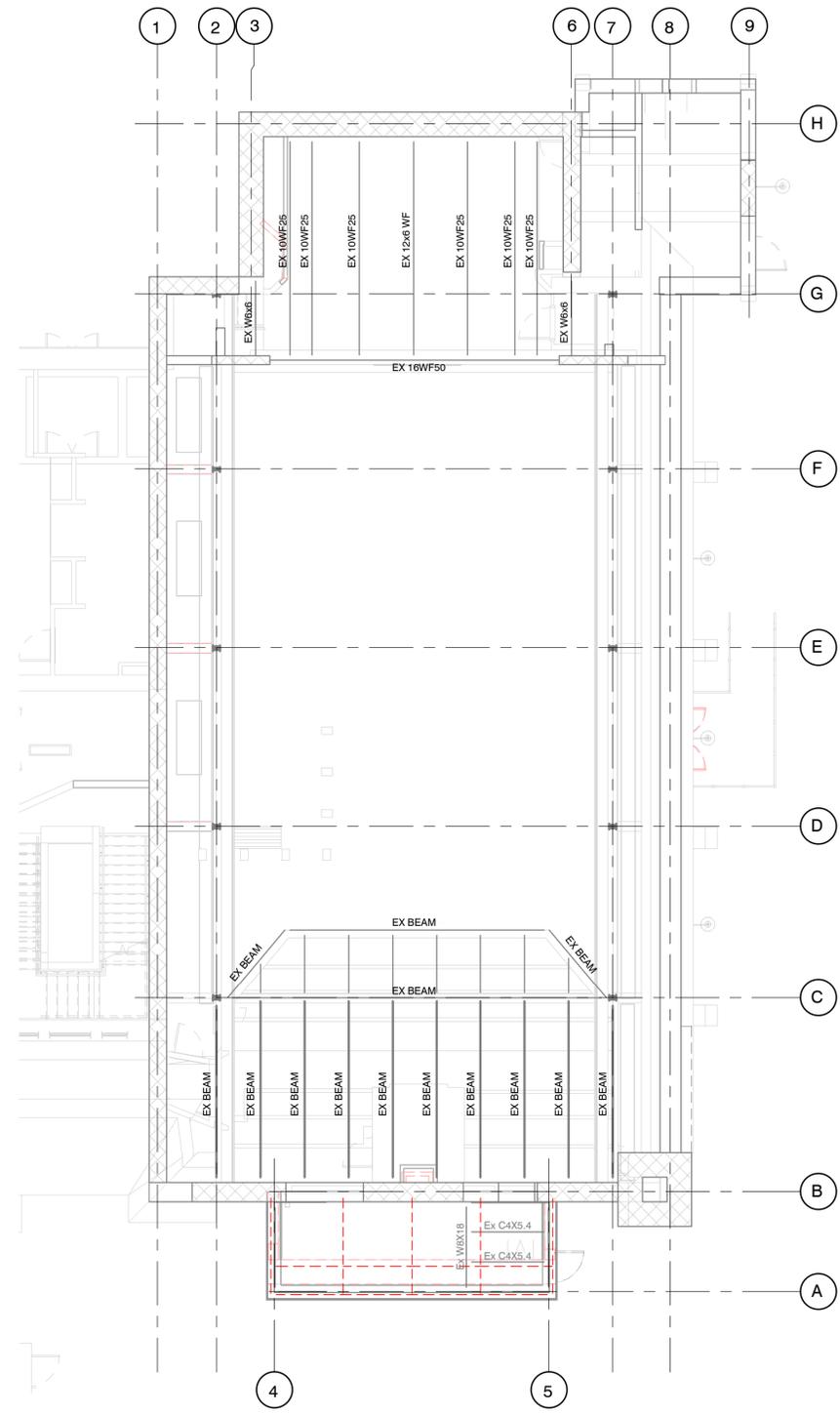
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 FRAMING PLANS

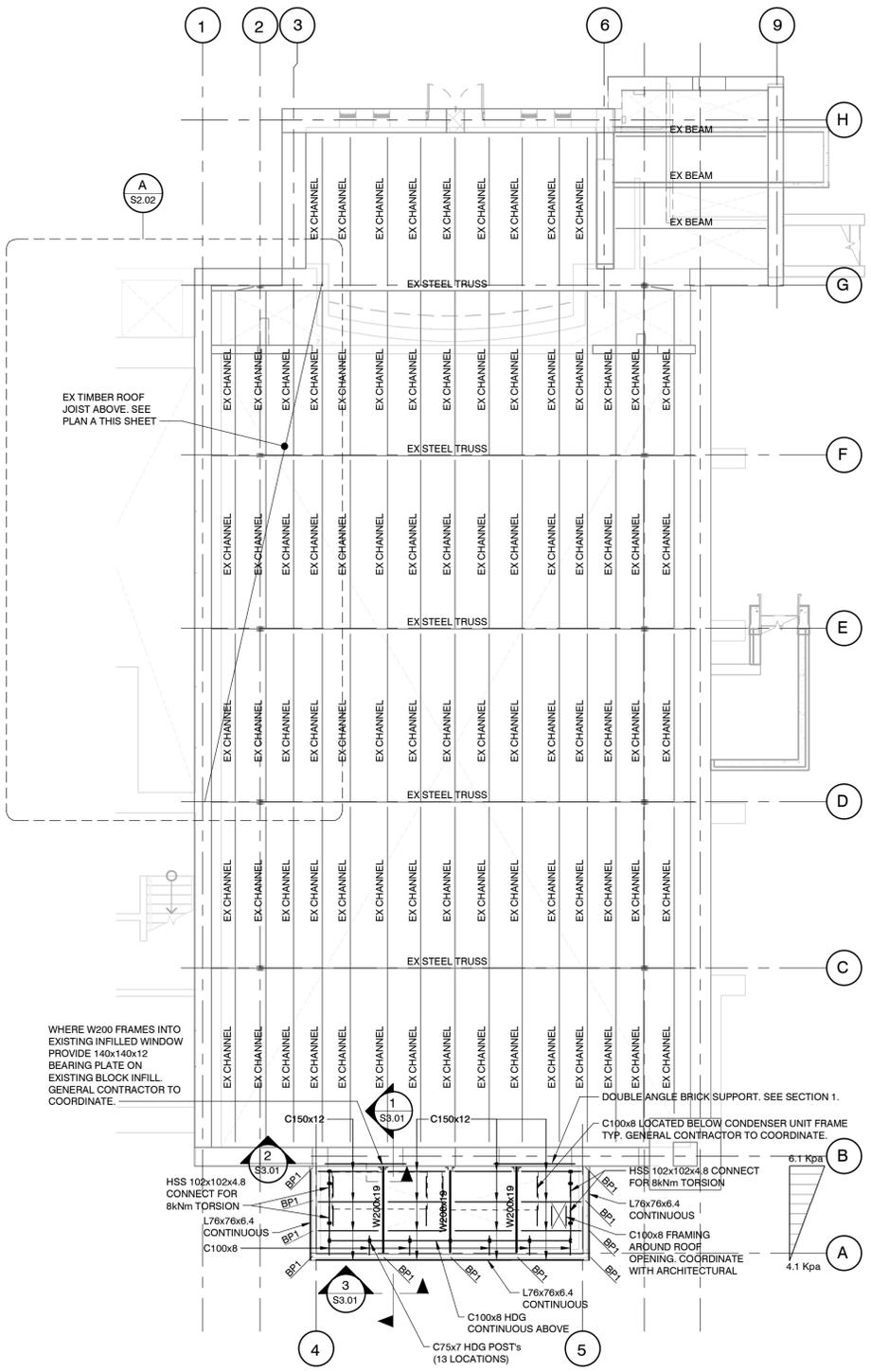
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S2.02



T/O STAGE OPENING

1 : 100



MAIN ROOF & SNOW DECK

1 : 100

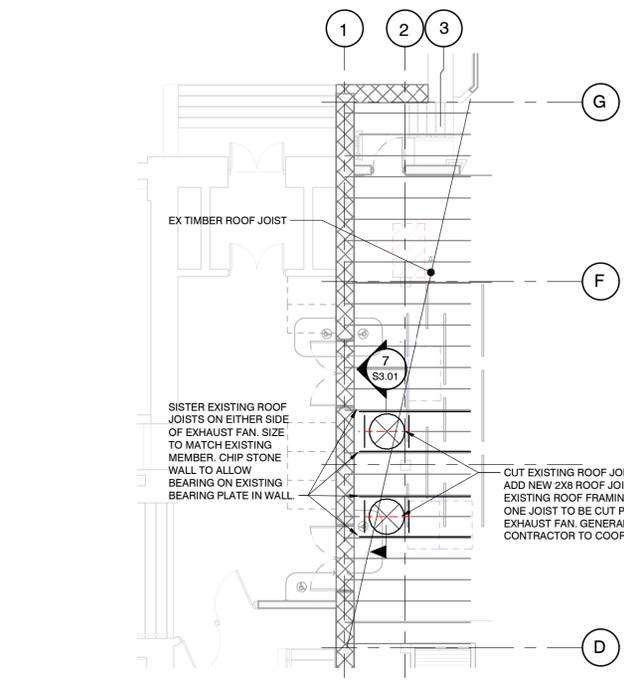
SNOW DECK NOTES

NEW SNOW DECK STRUCTURE DESIGNED FOR ROOF LOADING:
 DEAD LOAD = 1.4 KPA
 SNOW LOAD = 1.93 KPA

HIGH ROOF CONSTRUCTION TO BE 38mm DEEP STEEL ROOF DECK WITH FLUTES AT 152mm ON CENTRE, MINIMUM 0.91 NCT (CANAM 1.5B OR APPROVED ALTERNATE) SUPPORTED BY BEAMS SPACED AS SHOWN ON PLAN. WHERE POSSIBLE DECK TO BE 3 SPAN CONTINUOUS. REFER TO PLAN FOR AREAS OF THICKER NOMINAL CORE THICKNESS.

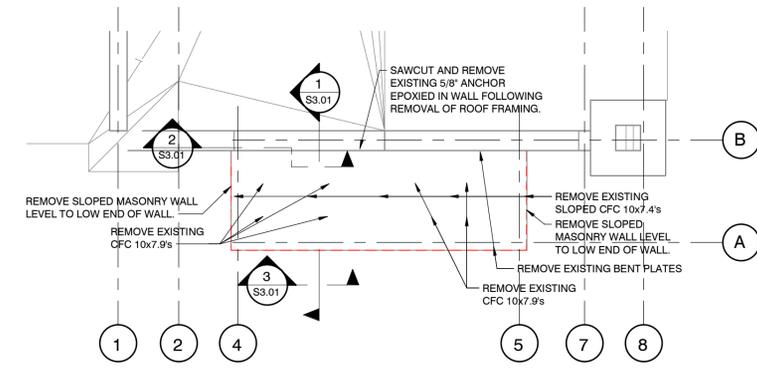
PERIMETER ANGLES, OR DOUBLE ANGLE MASONRY SUPPORTS SHOWN ON PLAN OR SECTION ARE TO BE CONTINUOUS UNLESS NOTED OTHERWISE. PROVIDE COMPLETE PENETRATION WELDS AT ALL SPLICES.

REFER TO NOTES ON S1.01 FOR ADDITIONAL DETAILS



UPPER ROOF FRAMING PLAN

1 : 100



MAIN ROOF DEMOLITION PLAN

1 : 100

DEMOLITION NOTES

- REFER TO DEMOLITION SPECIFICATIONS AND DETAILS PREPARED BY ARCHITECT FOR ADDITIONAL DETAILS AND REQUIREMENTS.
- THE DEMOLITION CONTRACTOR AND THEIR DEMOLITION ENGINEER ARE RESPONSIBLE TO REVIEW EXISTING INFORMATION AVAILABLE AND FIELD VERIFY ALL EXISTING STRUCTURAL CONDITIONS THAT ARE INFLUENCED BY THE DEMOLITION WORK OR AFFECT THE SAFETY OF THE CONSTRUCTION WORKERS, BUILDING OCCUPANTS AND BUILDING SERVICES.
- THE DEMOLITION CONTRACTOR AND THEIR DEMOLITION ENGINEER ARE RESPONSIBLE FOR PREPARING AND SUBMITTING THEIR DEMOLITION PLANS AND PROCEDURES REQUIRED TO OBTAIN THE DEMOLITION PERMIT. THE DEMOLITION CONTRACTOR'S PROFESSIONAL ENGINEER IS REQUIRED TO BE THE DEMOLITION ENGINEER OF RECORD AND COMPLETE GENERAL SITE REVIEWS DURING DEMOLITION.
- ALL CONSTRUCTION ACTIVITIES ASSOCIATED WITH THE REMOVAL OF THE EXISTING BUILDING ELEMENTS ARE TO BE IN ACCORDANCE WITH ALL APPLICABLE CURRENT MUNICIPAL AND OR PROVINCIAL HEALTH AND SAFETY LEGISLATION, THE ONTARIO BUILDING CODE AND RELEVANT CANADIAN STANDARDS ASSOCIATION'S (CSA) STANDARDS.
- PRIOR TO COMMENCING DEMOLITION, THE CONTRACTOR IS TO PREPARE AND SUBMIT DEMOLITION PLANS AND PROCEDURES, AND ENGINEERED SHOP DRAWINGS FOR SHORING SIGNED AND SEALED BY A PROFESSIONAL ENGINEER. OBTAIN ALL NECESSARY LICENSES AND PERMITS. THE GENERAL CONTRACTOR IS RESPONSIBLE TO PROVIDE ALL SHORING REQUIRED TO COMPLETE THE WORK INDICATED AND TO PREVENT DAMAGE TO THE BUILDING AND ENSURE SAFETY OF THE WORKERS.
- MEANS AND METHODS OF DEMOLITION SHALL BE COMPLETED IN A MANNER THAT DOES NOT DAMAGE EXISTING STRUCTURES THAT ARE TO REMAIN.
- REFER TO EXISTING DRAWINGS FOR MEMBER SIZES AND DETAILS.
- MTE CONSULTANTS IS NOT THE DEMOLITION ENGINEER OF RECORD.

