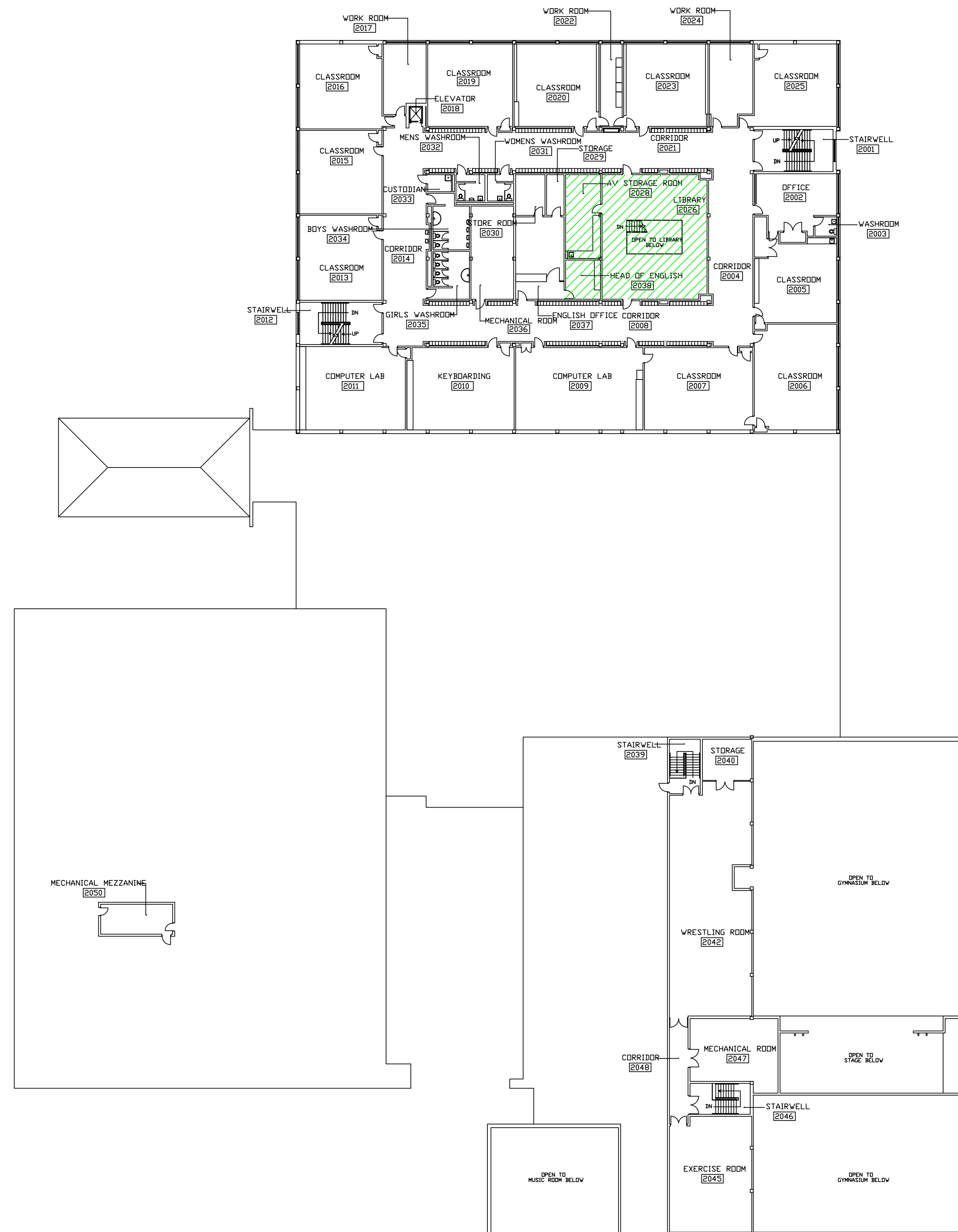
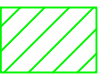
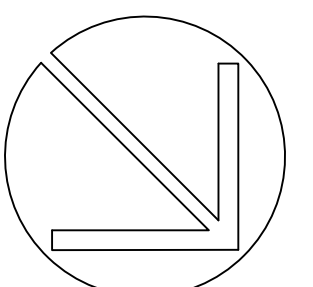


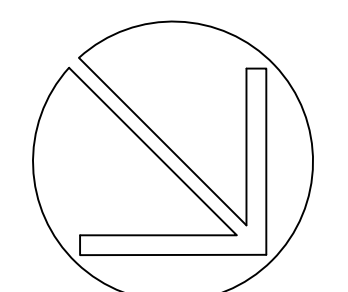
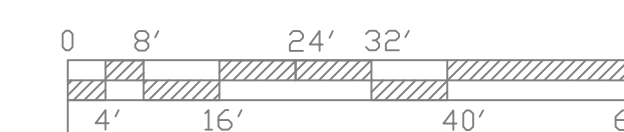
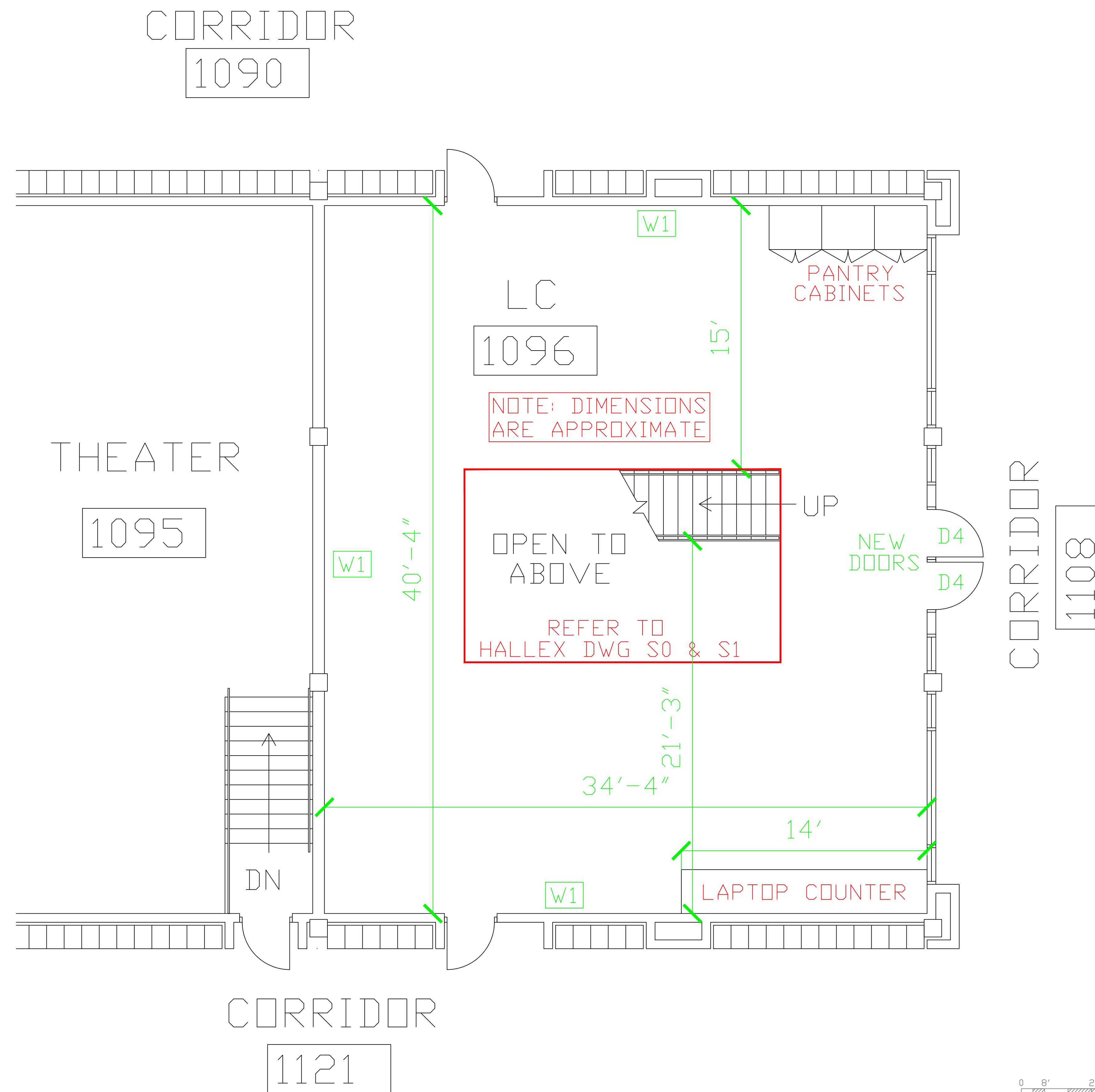
TENDER DRAWINGS

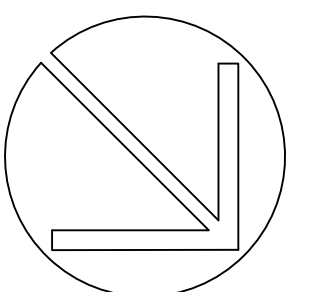
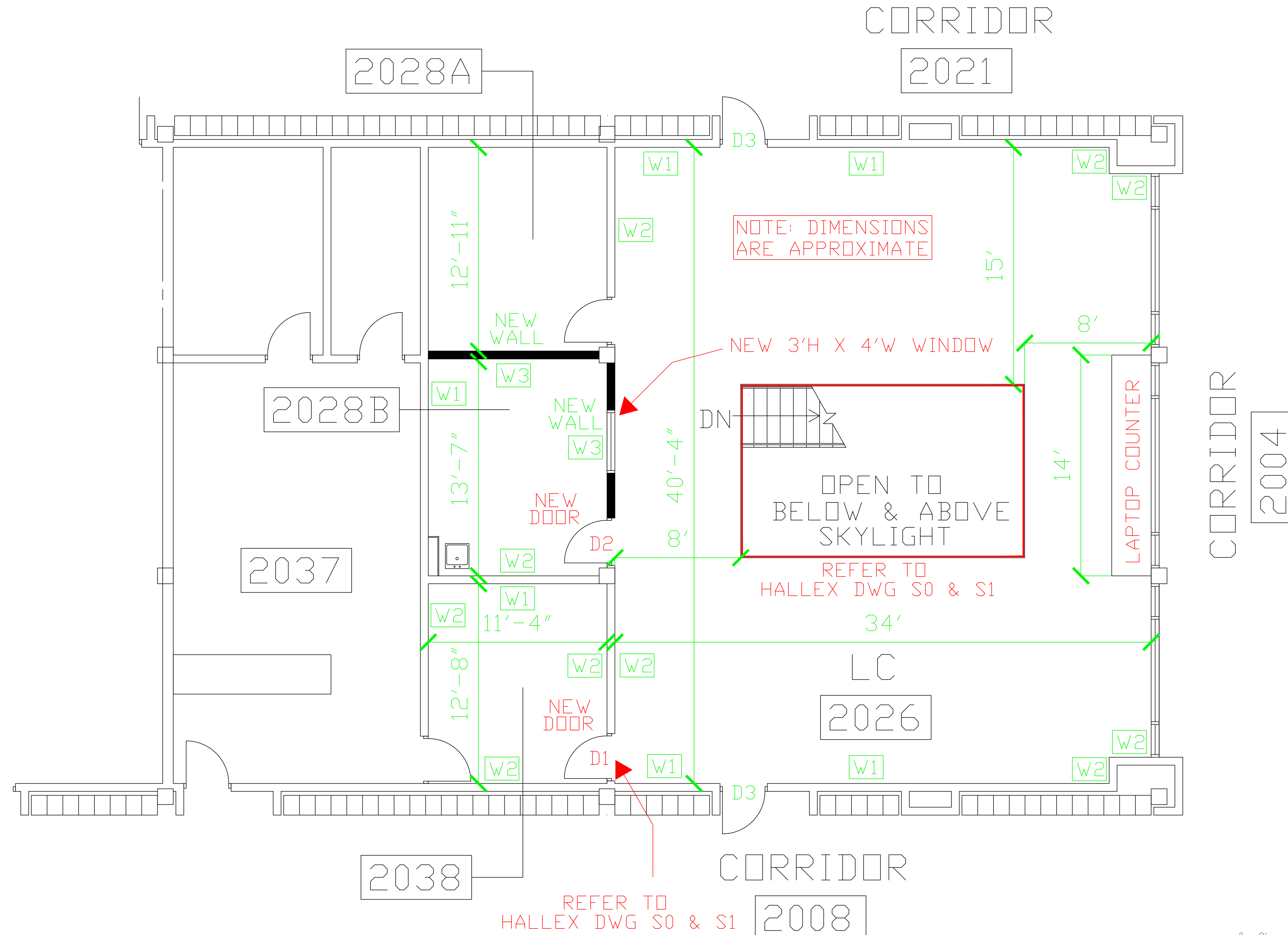
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AREA OF WORK 





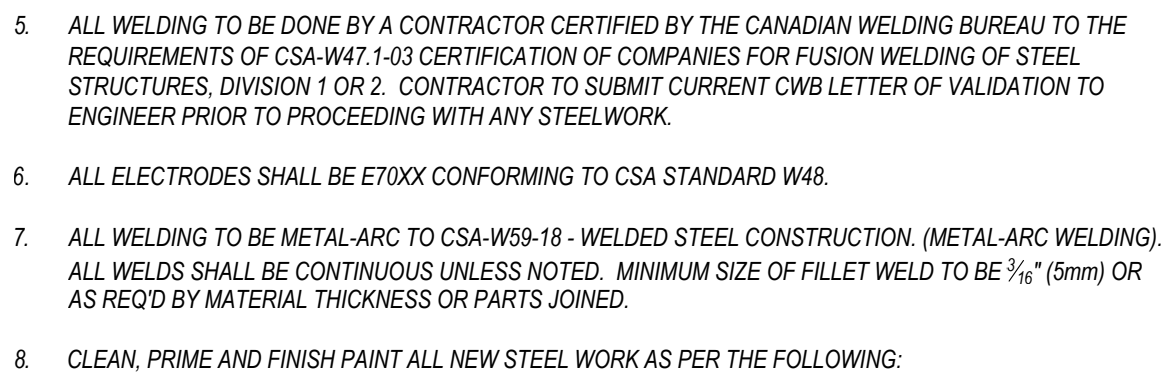


1. READ ALL STRUCTURAL/CIVIL DRAWINGS IN CONJUNCTION WITH ALL CONTRACT DOCUMENTS, INCLUDING REFERENCED ELECTRICAL, MECHANICAL, VENDOR DRAWINGS, AND SPECIFICATIONS
2. THE CONTRACTOR FOR ANY PORTION OF WORK SHALL VISIT THE SITE AND SHALL BE THOROUGHLY FAMILIAR WITH ALL THE PHYSICAL FEATURES THAT MAY AFFECT THE WORK IN ANY WAY.
3. FIELD MEASURE AND MAKE ADJUSTMENTS TO SUIT EXISTING CONDITIONS.
4. THE CONTRACTOR SHALL KEEP WORK SITES CLEAN AND FREE OF ALL CONSTRUCTION DEBRIS DURING THE PROCESS OF CONSTRUCTION AND LEAVE THE SITE CLEAN UPON COMPLETION OF WORK OR PORTIONS OF THE WORK.
5. CONSULTANT MUST APPROVE ALL DEVIATIONS FROM THE WORKING DRAWINGS. THE CONTRACTOR MUST KEEP AN ACCURATE RECORD OF ALL CHANGES FROM THE ORIGINAL INFORMATION SHOWN ON THE CONSTRUCTION DRAWINGS.
6. FEATURES OF CONSTRUCTION NOT FULLY SHOWN ARE OF THE SAME CHARACTER AS THOSE NOTED FOR SIMILAR CONDITIONS.
7. ALL CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING:
OCCUPATIONAL HEALTH AND SAFETY ACT
ONTARIO REGULATION 213/91 - CONSTRUCTION PROJECTS
THE ONTARIO BUILDING CODE AND THE NATIONAL BUILDING CODE
8. IF DISCREPANCIES EXIST BETWEEN THESE DWGS. AND THE SPECIFICATIONS, CONTACT ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING.
9. DO NOT SCALE THESE DRAWINGS.

1. DELIVER MATERIALS TO JOB SITE IN DRY CONDITION. KEEP MATERIALS DRY AND CLEAN UNTIL USE.
2. BACKFILL WITH GRANULAR A UNLESS OTHERWISE NOTED ON THE DRAWINGS. COMPACT IN ONE FOOT LIFTS TO A MINIMUM 95% P.D. TO SIX INCHES BELOW TOP OF CONCRETE.
3. ALL CEMENT TO BE PORTLAND BLAST FURNACE SLAG CEMENT TO CAN-A326 BLENDED HYDRAULIC CEMENTS CONSISTING OF 75% NORMAL TYPE 1 PORTLAND CEMENT AND 25% CEMENTITIOUS HYDRAULIC SLAG.
4. ALL REINFORCING STEEL TO BE GRADE 400 DEFORMED BARS TO CAN/CSA G30.18.
5. ALL BASE GROUT TO BE "MEADOWS V1" BY W.R. MEADOWS OR APPROVED EQUAL.
6. EPOXY GROUT FOR DOWELS TO BE ANCHORFIX ZCA BY SIKKA CANADA OR APPROVED EQUAL.
7. DISPOSAL OF ALL EXCAVATED MATERIAL SHALL BE OFF-SITE OTHER THAN APPROVED BACKFILL

A.B.	ANCHOR BOLT	L.L.H.	LONG LEG HORIZONTAL
A.F.F.	ABOVE FINISHED FLOOR	L.L.V.	LONG LEG VERTICAL
ALT.	ALTERNATE	L.S.H.	LONG SIDE HORIZONTAL
ARCH.	ARCHITECTURAL	L.S.V.	LONG SIDE VERTICAL
B.C.E.	BOTTOM CHORD EXTENSION	MANUF.	MANUFACTURER
B.F.F.	BELOW FINISHED FLOOR	MAX.	MAXIMUM
B.L.L.	BOTTOM LOWER LAYER	MECH.	MECHANICAL
BOT.	BOTTOM	MIN.	MINIMUM
B.U.L.	BOTTOM UPPER LAYER	MIR.	MIRROR
C.A.	COLUMN ABOVE	N.I.C.	NOT IN CONTRACT
C.B.	COLUMN BELOW	N.L.B.	NON-LOAD BEARING
CANT.	CANTILEVER	N.S.	NEAR SIDE
C.I.P.	CAST IN PLACE	N.T.S.	NOT TO SCALE
CONC.	CONCRETE	O/C	ON CENTER
CONT.	CONTINUOUS	OPP.	OPPOSITE
C.P.	COMPLETE PENETRATION	O.W.S.J.	OPEN WEB STEEL JOIST
c.c.	CENTER TO CENTER	PL.	PLATE
C.L.	CENTER LINE	P.L.A.	POINT LOAD
CRS	COURSE	P.L.A.	POINT LOAD ABOVE
c/w	COMPLETE WITH	P.T.	PRESSURE TREATED
DET.	DETAIL	R.D.	ROOF DRAIN
D.L.	DEAD LOAD	R.W.	REINFORCED WITH
do	DO OVER (DITTO)	S.C.	SAW CUT CONTROL JOINT
DP	DEEP (DEPTH OF MEMBER)	S.D.L.	STEP DOWNED DEAD LOAD
DWG.	DRAWING	S.D.F.	STEP DOWN FOOTING
D.W.L.S.	DOWELS	SECT.	SUPERSECTION
E.A.	EACH	SIM.	SIMILAR
E.E.	EACH END	S.J.	SNOW LOAD
E.F.	EACH FACE	S.O.G.	SLAB ON GRADE
EL.	ELEVATION	SPEC.	SPECIFICATIONS
ELEV.	ELEVATION	SYM.	STAGGERED
ELEC.	ELECTRICAL	STR.	STIRRUPS
EMBED.-	EMBEDMENT	SYG.	SYMMETRICAL
E.S.	EACH SIDE	THK.	THICK
E.W.	EACH WAY	THRU	THROUGH
EX.	EXISTING	T.L.L.	TOP LOWER LAYER
EXIST.	EXISTING	TYP.	TYPICAL
EXT.	EXTERIOR	T&B	TOP & BOTTOM
EXP. JT.-	EXPANSION JOINT	T&G	TONGUE & GROOVE
F.D.	FLOOR DRAIN	T.J.	TIE JOISTS
F.S.	FAR SIDE	T/O	TOP OF
FTG.	FOOTING	T.O.S.	TOP OF STEEL
GALV.	GALVANIZED	T.U.L.	TOP UPPER LAYER
GL.	GRID LINE	U.N.O.	UNLESS NOTED OTHERWISE
GRAN.	GRANULAR	ULS.	ULTIMATE LIMIT STATE
HK.	HOOK	SLS	SERVICEABILITY LIMIT STATE
H&V	HORIZONTAL & VERTICAL	UIS	UNDERSIDE
HORIZ.	HORIZONTAL	VERT.	VERTICAL
I.J.	ISOLATION JOINT	W.P.	WORK POINT
INT.	INTERIOR		
JT.	JOINT		
L.	LONG		
L.B.	LOAD BEARING		
L.L.	LIVE LOAD		

1. ALL WORK SHALL CONFORM TO CSA STANDARD S16 (LATEST EDITION) "LIMIT STATES DESIGN OF STEEL STRUCTURES": CONTRACTOR TO FABRICATE TO APPROVED SHOP DWGS. ONLY.
2. STEEL SHALL CONFORM TO CSA STANDARD G40.20 "GENERAL REQUIREMENTS FOR ROLLED OR WELDED STRUCTURAL QUALITY STEEL" AND TO CSA STANDARD G40.21 "STRUCTURAL QUALITY STEELS" TO THE FOLLOWING GRADES:
 HSS AND FLANGE SECTIONS - 350W (50 ksi)
 ALL OTHER HOT ROLLED SHAPES - 300W (44W)
 PLATES NOT GREATER THAN 1 1/2" THICKNESS - 300W (44W)
3. STRUCTURAL BOLTS TO BE ASTM F3125/F3125M LATEST EDITION. HEAT TREATED 120/105 KSI MIN. TENSILE STRENGTH. COMPLETE WITH HARDENED WASHERS AND HEAVY HEX NUTS.
4. ALL ANCHOR RODS TO ASTM-F1554 GRADE 50. ALL EXTERIOR ANCHOR RODS TO BE HOT-DIP GALVANIZED.



- 8.1. 3. PREPARE SURFACE
PREPARE SURFACE TO MEET THE REQUIREMENTS OF THE SPECIFICATION. REMOVE ALL
CONTAMINATION, OIL, GREASE, AND OTHER SUBSTANCES. CLEAN ALL SURFACES
THOROUGHLY. REMOVE ALL OLD PAINT, COATINGS, AND OTHER MATERIALS.
REPAIR ALL CRACKS, Holes, AND OTHER DEFECTS. SMOOTH ALL SURFACES.
PRIME ALL SURFACES WITH AN APPROPRIATE PRIMER. APPLY TWO COATS OF
SEMI-GLOSS ENAMEL #4306 SERIES, 1.5 TO 2 MILS DRY FILM THICKNESS, COLours BY OWNER.
AFTER ERECTION, CLEAN ALL SURFACES. TOUCH-UP ANY DAMAGED OR UNPAINTED AREAS WITH
PRIMER AND TWO FINISH COATS.

- DO NOT PAINT CONNECTION SURFACES OR SURFACES TO BE WELDED.
10. SUBMIT SHOP DRAWINGS TO ENGINEER FOR APPROVAL.
- 10.1 ALL SHOP DRAWINGS SHALL BEAR A SEAL OF A REGISTERED PROFESSIONAL ENGINEER FROM THE PROVINCE OF ONTARIO.
- 10.2 THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS.
- 10.3 UNLESS OTHERWISE DETAILED OR SPECIFIED, BEAM CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL BENDING CAPACITY OF THE MEMBER IN ITS FULLY BRACED CONDITION FOR THE APPROPRIATE SPAN AS LISTED IN THE CSC HANDBOOK BEAM LOAD TABLES.
- 10.4 CONNECTIONS SHALL CONFORM TO THE DETAILS IN THE CSC HANDBOOK.
- 10.5 NON-STANDARD CONNECTIONS AND CONNECTIONS TO EXISTING STRUCTURAL MEMBERS SHALL BE DESIGNED AND BEAR THE SEAL OF A PROFESSIONAL ENGINEER.
- 10.6 BEAM END CONNECTION DETAILS SHOWN ON THESE DRAWINGS ARE INTENDED TO SERVE AS A GUIDE ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL FINAL DIMENSIONS.
11. BEAM END CONNECTION DETAILS SHOWN ON THESE DRAWINGS ARE INTENDED TO SERVE AS A GUIDE ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL FINAL DIMENSIONS.
12. WHEN NON-AIRTIGHT HSS SECTIONS ARE EXPOSED TO THE WEATHER OR TO TEMPERATURE CHANGES THAT CAN CAUSE INTERIOR CONDENSATION, THE STEEL CONTRACTOR SHALL ENSURE ALL HSS SECTIONS HAVE TWO WEEP HOLES AT THE BASE OF COLUMN ABOVE THE BASEPLATE. THE WEEP HOLES SHALL BE LOCATED IN THE CENTER OF THE HSS FACE (ON OPPOSITE SIDES). FOR HSS SIZES 3" DIAMETER (OR SQUARE) AND LESS, PROVIDE 5/16" DIAMETER HOLE. FOR ALL OTHER SIZES PROVIDE 3/8" DIAMETER HOLE. VENT WEEP HOLE SIZE AND LOCATION FOR HOT-DIP GALVANIZING SHALL BE AS PER MANUFACTURER AND SHOULD BE COORDINATED WITH THE STEEL FABRICATOR. TOP VENT HOLES TO BE PLUGGED UPON COMPLETION OF HOT-DIP GALVANIZING U.N.O.
13. ALL EXTERIOR STEEL THAT IS EXPOSED TO WEATHER INCLUDING HANDRAILS AND GUARDS SHALL BE HOT-DIP GALVANIZED. THE CONTRACTOR SHALL ENSURE THE STEEL IS GALVANIZED TO EXPOSED STEEL FINISH STANDARDS (UNIFORM VISUAL APPEARANCE, FREE OF IMPERFECTIONS).
14. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS PER VERTICAL LINE IN EACH CONNECTED PIECE AND BE DESIGNED AS BEARING CONNECTIONS U.N.O.
15. U.N.O. COLUMN CAP PLATES SHALL BE 1/4" THICK AND COLUMN BASE PLATES SHALL BE 1/2" MINIMUM THICK.
16. DESIGN DRAWINGS INCLUDE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. SEE ALSO STRUCTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS AND DETAILS. WHERE ELEVATIONS, FOOT SLOPES, ETC. ARE SHOWN ON THE STRUCTURAL DRAWINGS, THEY MUST BE CONFIRMED WITH THE ARCHITECTURAL DRAWINGS.
17. MISCELLANEOUS MATERIALS AND ACCESSORIES ASSOCIATED WITH GOOD PRACTICE THAT ARE NOT SHOWN SHALL BE PROVIDED.
18. U.N.O. DO NOT OVERSIZE HOLES IN STEEL TO FIT ANY ANCHOR LOCATIONS. FOR COLUMN BASE PLATE HOLES, UNLESS NOTED OTHERWISE ON DRAWINGS, SEE TABLE BELOW.

19. PROVIDE MINIMUM $L2 \times 2 \times \frac{3}{16}$ TRIMMER ANGLE (U.N.O. ON DRAWING) FOR STEEL DECK SUPPORT TYPICAL AROUND PERIMETER OF BUILDING, WELDED TO OWSJ AND/OR STEEL BEAM.

20. THE CONTRACTOR WILL EMPLOY A QUALIFIED PERSON TO INSPECT THE CONNECTIONS, INCLUDING BOLT TENSION AND WELDING, IN ACCORDANCE WITH CSA S16.19 SECTIONS 23 AND 30.

1. THE DESIGN AND FIELD REVIEW OF FORMWORK, SHORING AND RE-SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR. RE-SHORING DRAWINGS SHALL BE SUBMITTED TO HALLEX ENGINEERING LTD. FOR THE EFFECT ON THE BASE BUILDING STRUCTURE ONLY.
2. NO COLUMN OR WALL FORMS SHALL BE REMOVED BEFORE CONCRETE HAS REACHED 10 MPa FOR ARCHITECTURAL CONCRETE OR 8 MPa FOR OTHER COLUMNS OR WALLS.
3. NO SLAB FORMS OR BEAM FORMS SHALL BE REMOVED BEFORE CONCRETE HAS REACHED 75% OF THE 28 DAY STRENGTH BEFORE STRIPPING/RE-SHORING.
4. ALL SLABS, BEAMS, GIRDERS ETC. TO BE SHORED UNTIL CONCRETE REACHED DESIGN STRENGTH.
5. NO CONCRETE MAY BE REMOVED WITH PERCUSSIVE METHODS SUCH AS CHIPPING JACK-HAMMERING WITHOUT PRIOR APPROVAL OF HALLEX ENGINEERING LTD.

1. STUD DESIGN IS BASED ON CANADIAN SHEET STEEL BUILDING INSTITUTE LIGHT WEIGHT STEEL FRAMING WALL STUD & FLOOR JOIST LOAD TABLES CSSBI 58-2004, AND WITH CAN - S136-01.
2. ALL METAL STUDS SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF CSA STANDARD CAN3-S136.
3. ALL METAL STUDS SHALL BE TO ASTM 446 STANDARD SPECIFICATIONS FOR SHEET STEEL, ZINC COATED (GALVANIZED) BY THE HOT-DIP PROCESS, STRUCTURAL QUALITY.
4. ALL STUD SCREWS SHALL BE ZINC COATED AND GALVANIZED BY HILTI OR BAILEY. BLACK SCREWS ARE NOT ACCEPTABLE.
5. PARTITIONS AND NON LOAD BEARING WALLS TO HAVE A 2 7/8" LONG LEG SLOTTED TOP TRACK TO PREVENT TRANSFER OF LOAD FROM ABOVE.
6. CONTRACTOR SHALL SUPPLY TO THE ENGINEER SHOP DRAWINGS SHOWING ALL LOAD-BEARING WALL AND FLOOR SYSTEMS COMPLETE WITH CONNECTION DETAILS OF BRIDGING, HEADERS, BRACING AND LATERAL SUPPORTS.
7. CONTRACTOR MAY CONSULT THE LIGHTWEIGHT STEEL FRAMING MANUAL PRODUCED BY THE CANADIAN SHEET STEEL BUILDING INSTITUTE (CSSBI) FOR STANDARD CONNECTION DETAILS.
8. DESIGN WIND PRESSURE FOR THE FOLLOWING WIND LOADING LEVELS:
 - a) GROUND FLOOR TO ROOF = 25 PSF (1.2 kPa) UNFACTORED FOR STRENGTH AND DEFLECTION.
9. FASTEN ALL LIGHT GAUGE METAL TRACKS TO CONCRETE WITH HILTI METAL HIT ANCHORS 1/2" Ø x 1 1/2" LONG @ 16" O.C. STAGGERED.
10. FASTEN ALL LIGHT GAUGE METAL TRACKS TO STRUCTURAL STEEL WITH THE FOLLOWING HILTI PINS:
 - i) FOR BASE STEEL THICKNESSES BETWEEN 3/8" AND 5/8" USE HILTI ENP2K-20-15 DRIVE PINS 3.7mm Ø AND 15mm Ø DOUBLE WASHER ATTACHED WITH MODEL DX750 FASTENING TOOL, 2 SPACED AT 12" MINIMUM SPACING AND 3/4" MIN EDGE DISTANCE.
 - ii) FOR BASE STEEL THICKNESSES ABOVE 5/8" USE HILTI ENP2-21-15 DRIVE PINS 4.5mm Ø AND 15mm Ø DOUBLE WASHER ATTACHED WITH A MODEL DX750 FASTENING TOOL, 2 SPACED AT 12" MINIMUM SPACING AND 3/4" MIN EDGE DISTANCE.
11. ALL SCREWS ARE No. 8-16 BULDEX 'TENS' SELF TAPPING SELF DRILLING D=0.135" ROOT DIAMETER OF THREADED PORTION (MINIMUM) U.K.O.
12. ALL STUDS AND ACCESSORIES SHALL HAVE A MINIMUM COATING OF Z275 IN ACCORDANCE WITH ASTM A525M-87.
13. ALL STUDS ARE MINIMUM 18 GAUGE @ 16" C/C U.N.O.
14. ALL TRACKS ARE 18 GAUGE UNLESS NOTED OTHERWISE.
15. STEEL STUD CONTRACTOR IS TO CALL HALLEX ENGINEERING LTD. FOR INSPECTION OF STEEL STUD FRAMING PRIOR TO ANY INSULATING DRYWALLING OR CLADDING ACTIVITIES.
16. STEEL STUDS ARE DESIGNED FOR A MAXIMUM DEFLECTION OF 1/720 UNDER WIND LOAD OF 25 PSF UNLESS NOTED OTHERWISE
17. STEEL STUD WALLS AND EXTERIOR WALL SHEATHING ARE NOT DESIGNED TO SUPPLY LATERAL STABILITY TO THE BUILDING.
18. PROVIDE BRIDGING FOR LOAD-BEARING WALLS AT 4'-0" MAXIMUM VERTICAL SPACING USING STANDARD BRIDGING CHANNELS (1 1/2" X 1 1/2" X 0.0566"). ATTACH CHANNELS TO STUDS WITH STANDARD ANGLE CLIPS (1 1/2" X 1 1/2" X 3/8" LONG). SECURE EACH END OF BRIDGING TO STEEL COLUMNS OR MASONRY WALLS.
19. ENSURE CUT OUTS ARE 12" MINIMUM AWAY FROM CONNECTIONS TOP AND BOTTOM OF STUD SPAN.
20. SCREW BUILT-UP STUD SECTIONS TOGETHER WITH #8-16 SCREWS WITH A VERTICAL SPACING OF 12" O.C. U.N.O.
21. DO NOT ATTACH LOOSE UNITS TO STEEL STUDS.
22. PROVIDE BRIDGING FOR LOAD-BEARING WALLS AT 4'-0" MAXIMUM VERTICAL SPACING USING STANDARD BRIDGING CHANNELS (1 1/2" X 1 1/2" X 0.0566"). ATTACH CHANNELS TO STUDS WITH STANDARD ANGLE CLIPS (1 1/2" X 1 1/2" X 3/8" LONG).

1. ENSURE PROPER FASTENING TOOL IS USED TO ACHIEVE MINIMUM PENETRATION
2. MIN. CONC. STRENGTH ASSUMED = 20 MPa.

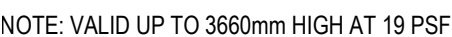
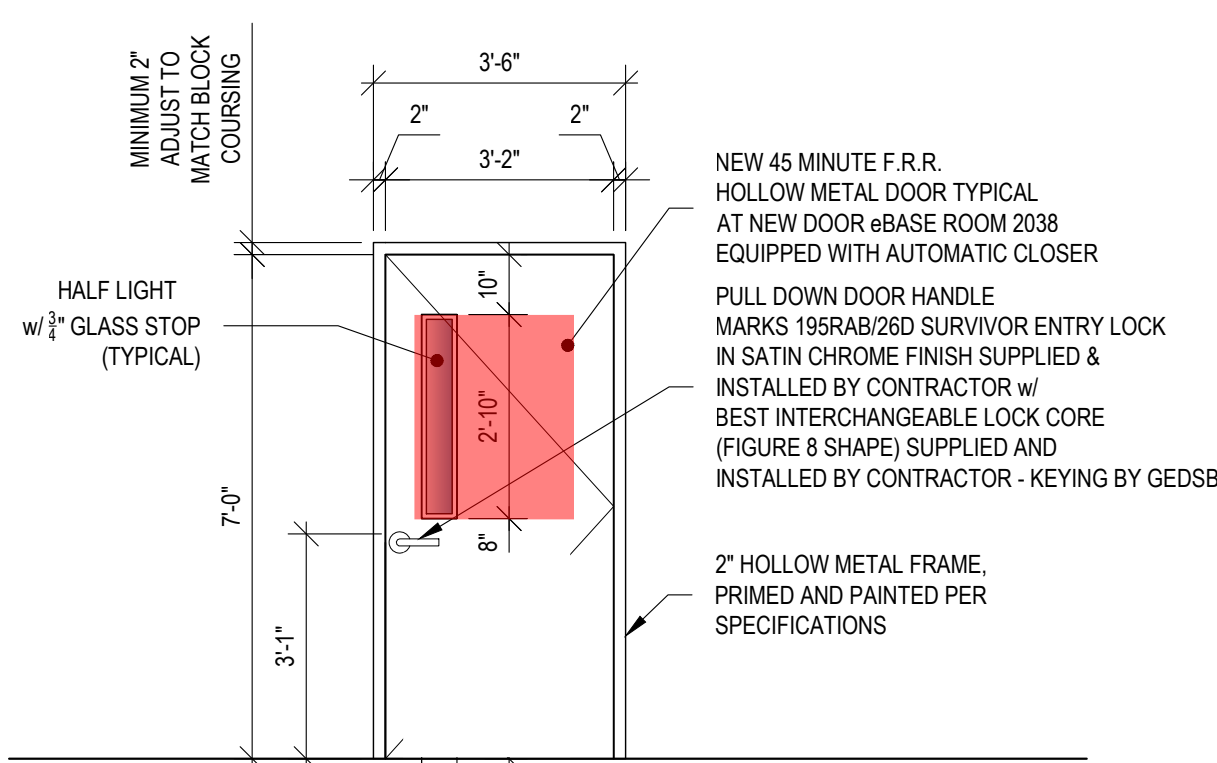


Diagram illustrating the installation of a bridging channel using an L38 x 38 x 89mm long clip and #8-16 screws into a pre-punched cut out in a steel stud.

- STEEL STUD (REFER TO PLAN FOR SIZE, SPACING, AND GAUGE)
- PRE-PUNCHED CUT OUT
- (2) #8-16 SCREWS INTO STUDS
- L38 x 38 x 89mm LONG CLIP
- (2) #8-16 SCREWS INTO BRIDGING CHANNEL
- BRIDGING CHANNEL

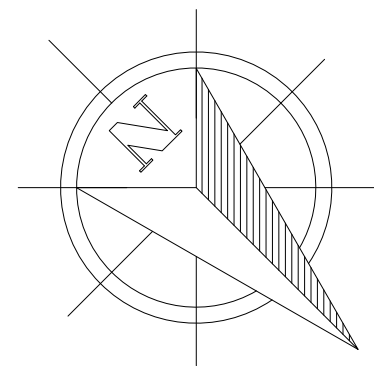
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SCALE: N.T.S.



DOOR SCHEDULE												
MARK	ROOM	SIZE	FRR RATING	DOOR		FRAME		VIEWER	HANDLE	SELF CLOSER	HINGES	DOOR STOP
				MATERIAL	FINISH	MATERIAL	FINISH					
D1	HEAD OF ENGLISH - 2038	NEW 3'-2" x 7'-0"	45 MINUTE	HOLLOW METAL	PAINT	HOLLOW METAL	PAINT	HALF LIGHT , 2'-0" WIDE x 2'-10" HIGH w/ 6mm TEMPERED GLASS	NEW MARKS 195 RAB126D SURVIVOR ENTRY LOCK	INGERSOLL RAND LCN 1460 SERIES	STANLEY SECURITY SOLUTIONS CB179 NON REMOVABLE PINS	INGERSOLL RAND STANDARD METAL 'S' SERIES FLOOR AND WALL STOPS IN DULL CHROMIUM FINISH

S0 - GENERAL NOTES , KEY PLAN, DOOR SCHEDULE
AND DRAWING LIST
S1 - PARTIAL FIRST AND SECOND FLOOR PLANS AT
LIBRARY, SECTIONS AND DETAILS



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Niagara Falls, ON L2E 4C9
Tel: 905-357-4015 Fax: 905-353-1105

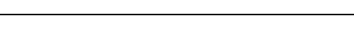
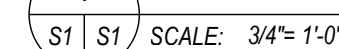
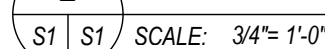
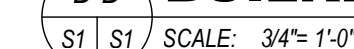
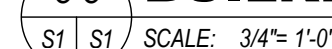
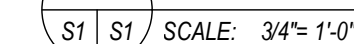
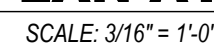
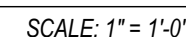
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**GRAND ERIE DISTRICT
SCHOOL BOARD
349 ERIE AVENUE
BRANTFORD, ON
N3T 5V3**

**LIBRARY RENOVATIONS
HAGERSVILLE SECONDARY
SCHOOL
70 PARKVIEW ROAD
HAGERSVILLE, ON
N0A 1H0**

SHEET TITLE:
**GENERAL NOTES,
KEY PLAN, DOOR SCHEDULE
AND DRAWING LIST**

JOB NUMBER: 251003	
DATE: OCT. 9, 2025	
DRAWN BY: AJ	
DESIGNED BY: AJ	
CHECKED BY: MLJH	
SCALE: AS SHOWN	
DWG. S0	REV. 0



1. SITE CONFIRM ALL DIMENSIONS AND CONDITIONS.
2. SEE DRAWING S0 FOR GENERAL NOTES.

4999 Victoria Ave.,
 Para Falls, ON L2E 4C9
 57-4015 Fax: 905-353-1105

Tel: 9