GENERAL NOTES

- 1. CHECK ALL DIMENSIONS ON THESE DRAWINGS WITH ALL OTHER DRAWINGS, INCLUDING BUT NOT LIMITED TO DRAWINGS PREPARED ARCHITECTURAL. MECHANICAL OR ELECTRICAL CONSULTANTS. REPORT ANY INCONSISTENCIES TO THE ARCHITECT OR ENGINEER PRIOR TO COMMENCING WITH THE WORK.
- DO NOT SCALE THE DRAWINGS. 2. THE DESIGN LIVE LOADS ARE INDICATED ON THE DRAWINGS. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LOADS.
- 3. THE COMPLETED STRUCTURE IS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING AND ANY
- SUPPORT OF EXISTING OR ADJACENT STRUCTURES AS REQUIRED. ALL BRACING AND SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR. 4. CONSTRUCTION FEATURES NOT FULLY SHOWN ARE COMPARABLE TO SIMILAR CONDITION DETAILS.

OTHER TEMPORARY OR PERMANENT MEASURES AS REQUIRED DURING

CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY

- 5. REFER TO OTHER CONSULTANTS DRAWINGS FOR DETAILS OF OPENINGS, PITS, CHAMFERS, DEPRESSIONS NOT INDICATED ON THE STRUCTURAL
- 6. ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE LATEST ONTARIO BUILDING CODE, LATEST APPLICABLE REGULATIONS, AND GOOD CONSTRUCTION PRACTICES.
- 7. THE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS. 8. CLARIFY ANY QUERIES WITH THE ENGINEER REGARDING THE INTERPRETATION

OF THE DRAWINGS, PRIOR TO THE COMMENCEMENT OF ANY WORK.

MASONRY NOTES 1. ALL STRUCTURAL ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD S304.1. ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH CSA STANDARD A371. ALL MASONRY CONNECTORS. REINFORCING AND TYING SHALL BE IN ACCORDANCE WITH CSA A370. ALL

2. ALL CONCRETE BLOCKS SHALL BE NORMAL WEIGHT TYPE H/15/A/M UNLESS OTHERWISE NOTED. MORTAR SHALL BE TYPE S FOR LOADBEARING AND TYPE N FOR NON-LOADBEARING. 3. VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 6000mm. REFER TO ARCHITECTURAL DRAWING FOR DETAILS AND

MORTAR AND GROUT SHALL BE IN ACCORDANCE WITH A179.

- 4. TRIM ALL OPENINGS WITH 2-15M BARS. 5. GROUT SHALL CONSIST OF ON ONE PART PORTLAND CEMENT, THREE PARTS SAND (MAXIMUM AGGREGATE SIZE SHALL BE 10mm) WITH WATER TO PROVIDE A MINIMUM 10MPa COMPRESSIVE STRENGTH AT 28 DAYS.
- SLUMP SHALL BE 200mm TO 250mm. 6. ALL CELLS CONTAINING REINFORCING SHALL BE GROUTED SOLID. TWO BLOCK COURSES BELOW BEARING PLATES SHALL BE GROUTED SOLID. 7. THE MASONRY SHALL BE CONSTRUCTED EVENLY WITH MAXIMUM LIFTS OF
- 1200 PER DAY. DO NOT TOOTH AND BOND OR STACK BOND MASONRY. RAKE BACK ENDS OF UNFINISHED WALLS. 8. ALL MORTAR JOINTS SHALL BE TOOLED (CONCAVE). A MINIMUM BED JOINT OF 6mm IS REQUIRED FOR THE STARTING COURSE TO A MAXIMUM OF
- 20mm. THE BED JOINTS SHALL BE 10mm. 9. PROVIDE VERTICAL AND HORIZONTAL REINFORCING AS FOLLOWS UNLESS
- NOTED OTHERWISE ON THE DRAWINGS. - 140 CONCRETE BLOCK - HEAVY DUTY TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE. - 190 CONCRETE BLOCK - 15M VERTICAL AT 800 O.C. & HEAVY DUTY

TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE.

- 10. THE HORIZONTAL REINFORCING AT EXTERIOR WALLS SHALL BE GALVANIZED. DO NOT EXTEND HORIZONTAL REINFORCING THROUGH CONTROL JOINTS UNLESS OTHERWISE NOTED.
- 11. PROVIDE A STEEL LINTEL OVER ALL OPENINGS OR RECESSES INCLUDING OPENINGS FOR MECHANICAL AND ELECTRICAL COMPONENTS. ALL EXTERIOR LINTELS TO BE HOT DIP GALVANIZED.

WALLS IN CONSTRUCTION. PROTECT THE MASONRY WALLS FROM THE ELEMENTS AT ALL TIMES EXCEPT DURING CONSTRUCTION PROGRESS.

12. BUILD THE MASONRY SOLID AROUND ALL BEAM, LINTEL AND JOIST POCKETS. INSTALL BEARING PLATES AT THE SPECIFIED ELEVATION AND GROUT THE PLATE INTO THE WALL A MINIMUM OF 400mm. 13. PROVIDE TEMPORARY BRACING AS REQUIRED TO SUPPORT THE MASONRY

STRUCTURAL STEEL NOTES

- 1. ALL STRUCTURAL STEEL ELEMENTS, INCLUDING DESIGN OF ELEMENTS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH CAN/CSA S16. ALL STRUCTURAL STEEL SHALL CONFORM TO CSA G40.21 (300W) EXCEPT W SECTIONS AND PLATES G40.21 (350W), HSS MEMBERS G40.21 (350W) CLASS C OR ASTM A500 GRADE C, ANCHOR BOLTS ASTM A307, COLD FORMED SECTIONS ASTM A570M GRADE 350W. UNLESS OTHERWISE
- NOTED, ALL SECTIONS SHALL BE PRIME PAINTED WITH THE SURFACE PREPARATION AND PAINTING PROCEDURES IN ACCORDANCE WITH CAN/CGSB 85.10. ALL FIRE RATED STEEL TO BE LEFT UNPRIMED. 3. ALL WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH CAN/CSA W59. THE STEEL FABRICATOR SHALL BE FULLY QUALIFIED UNDER THE REQUIREMENTS BY THE CANADIAN WELDING BUREAU IN CONFORMANCE
- WITH CAN/CSA W47.1. 4. DESIGN ALL MOMENT AND SHEAR CONNECTIONS FOR THE FULL CAPACITY OF THE SMALLER MEMBER IN THE CONNECTION UNLESS OTHERWISE
- 5. PROVIDE MINIMUM BEARING LENGTH OF STEEL MEMBERS AS FOLLOWS: – ON MASONRY – 150mm
- ON STEEL 90mm 6. THE BASE PLATE AND BEARING PLATE GROUT SHALL BE OF THE CEMENTITIOUS NON-SHRINK TYPE.
- 7. FULLY WELD THE BASE PLATE TO THE COLUMN TO DEVELOP THE ANCHOR BOLTS. PROVIDE CAP PLATES ON ALL COLUMNS. PROVIDE 6mm CAP PLATES ON ALL COLUMNS. 8. PROVIDE MINIMUM 175x10x175 BEARING PLATES FOR ALL STRUCTURAL
- 9. ALL BOLTS SHALL BE TIGHTENED WITH A SUITABLE TORQUE WRENCH IN ACCORDANCE WITH CSA S16. 10. ALL STEEL EXPOSED TO THE EXTERIOR TO BE HOT DIP GALVANIZED. 11. ERECT STRUCTURAL STEEL IN ACCORDANCE WITH CSA S16 AND IN
- CONFORMANCE WITH THE APPROVED SHOP DRAWINGS.

STEEL c/w 2-150 ANCHORS UNLESS OTHERWISE NOTED.

METAL DECK NOTES 1. ALL WELDING OF DECK AND ASSOCIATED COMPONENTS SHALL BE CARRIED

OUT IN ACCORDANCE WITH CAN/CSA W59. THE STEEL FABRICATOR AND INSTALLER SHALL BE FULLY QUALIFIED UNDER THE REQUIREMENTS OF THE CANADIAN WELDING BUREAU (CWB) IN CONFORMANCE WITH CAN/CSA W47.1. 2. ALL DECK SHALL BE DESIGNED TO RESIST THE LOADS NOTED ON THE DRAWINGS IN ACCORDANCE WITH PART 4 OF THE LATEST ONTARIO BUILDING CODE. DECK AND FASTENING SHALL HAVE ADEQUATE CAPACITY TO SUPPORT THE DESIGN GRAVITY AND DIAPHRAGM LOADS AS NOTED ON THE DRAWINGS. 3. DECK SHALL BE EITHER 38mm (1 ½") OR 76mm (3") DEEP IN ACCORDANCE WITH CSA S136 AND SHALL BE FABRICATED FROM ASTM A653 SS OR GRADE 230 GALVANIZED STEEL WITH A ZF75 GALVANNEAL OR Z275 GALVANIZED ZINC COATING. THE MINIMUM NOMINAL STEEL CORE THICKNESS SHALL BE 0.76mm (22 ga).

4. DECK SHALL BE FASTENED TO THE SUPPORTING STRUCTURE WITH 20mm ARC SPOT WELDS AT NOT LESS THAN 300mm (12") O.C. (36/4) AT SUPPORTS AND NOT LESS THAN 150mm (6") O.C. (36/7) WITHIN 3m (10'-0") OF THE NEAREST BUILDING EDGE. SIDELAPS SHALL BE CLINCHED AT 600mm (24") O.C. ALL WELDS SHALL BE TOUCHED UP WITH PRIMER. MECHANICAL FASTENERS MAY ONLY BE USED WITH THE PERMISSION OF THE ENGINEER.

5.THE STEEL DECK CONTRACTOR SHALL REINFORCE THE DECK FOR ROOF OPENINGS FROM 150mm (6") TO 400mm (16") WIDE AND FOR FLOOR OPENINGS FROM 150mm (6") TO 300mm (12") WIDE WITH LIGHT GAUGE METAL ANGLES (MIN. 16 GA).

LIGHT GAUGE STEEL FRAMING NOTES SHORING NOTES

- 1. THESE NOTES APPLY TO THE STEEL STUD FRAMING COMPONENT OF THE EXTERIOR WALL SYSTEM ONLY. . THE DESIGN WIND LOADING IS 1.2kN/m² (25 PSF) DETERMINED BY
- O.B.C. REQUIREMENTS AND CAN-S136. DEFLECTION IS LIMITED TO 3. THE DESIGN OF FRAMING SYSTEM IS BASED ON PUBLISHED STUD SECTION PROPERTIES BY BAILEY METAL PRODUCTS LIMITED.
- MATERIAL 1. THE MINIMUM BASE METAL THICKNESS FOR ALL METAL WALL COMPONENTS, EXCLUDING COATINGS ARE NOTED ON THE DRAWINGS.
- 2. STEEL MEETS THE REQUIREMENTS OF A.S.T.M. A653/A653M SS GRADE 33 (230) FOR 1.22mm MATERIAL AND THINNER, AND SS GRADE 50 (340) CLASS 1 FOR 1.52mm MATERIAL AND THICKER.

3. GALVANIZING TO BE HOT-DIP PROCESS, G90 (Z275). EXECUTION

- 1. METHOD OF CONSTRUCTION SHALL BE BY STICK BUILDING ON SITE. 2. CONNECTIONS SHALL BE ACCOMPLISHED BY SELF DRILLING SCREWS AND OTHER FASTENERS AS SHOWN ON THESE DRAWINGS. PENETRATION BEYOND JOINED MATERIALS SHALL BE NOT LESS THAN THREE EXPOSED THREADS, ALL CONNECTORS USED IN ASSEMBLIES SHALL BE OF CORROSION RESISTANT MATERIAL COMPATIBLE WITH GALVANIZED COATINGS WITH A MINIMUM COATING THICKNESS OF 0.039mm ZINC OF
- CADMIUM PLATES. NO BLACK CONNECTORS WILL BE ACCEPTED. SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER. SCREWS COVERED BY SHEATHING MATERIALS SHALL HAVE LOW PROFILE
- 4. WIRE TYING IS NOT PERMITTED IN STRUCTURAL APPLICATIONS. 5. CUTTING OF STEEL FRAMING MEMBERS SHALL BE BY SAW OR SHEAR. NO TORCH OR MANUAL CUTTING IS PERMITTED.

6. SPLICING OF STUDS OR TRACK IS NOT PERMITTED EXCEPT AS NOTED

- BRIDGING SHALL BE OF SIZE, SPACING AND TYPE SHOWN ON THE DRAWINGS AND SHALL BE INSTALLED SO AS TO PROVIDE RESISTANCE
- TO MINOR AXIS BENDING AND ROTATION OF STUDS. PROVIDE BRIDGING 8. TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.
- STUDS SHALL SEAT INTO TOP AND BOTTOM TRACKS WITH THE GAP BETWEEN THE END OF THE STUD AND WEB OF THE TRACK NOT TO 10. VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/1000

REQUIREMENTS OF THE FINISHING MATERIALS.

11. HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/1000 OF THEIR RESPECTIVE LENGTHS. 12. SPACING OF STUDS SHALL BE WITHIN 3mm FROM DESIGN SPACING PROVIDED THAT CUMULATIVE ERROR DOES NOT EXCEED THE

CONCRETE NOTES

SLAB ON GRADE

- TOPPING

- 1. ALL FRAMES AND SHORING JACKS TO BE PLUMB AND LEVEL. 2. SHORING JACKS TO BE DESIGNED BY THE SUPPLIER FOR THE LOADS AND HEIGHTS SHOWN, INCLUDING BRACING.
- 3. MAX. EXTENSION OF SCREWJACKS WILL BE 400mm UNLESS NOTED. 4. SCAFFOLDING SHALL BE ERECTED IN ACCORDANCE TO C.S.A. CODE
- 5. SHORING TO REMAIN IN PLACE UNTIL BEAM AND ALL BRACING IS COMPLETELY INSTALLED. 6. CONTRACTOR TO PREPARE AND SUBMIT FULL SHORING DRAWINGS FOR APPROVAL FOR ALL TEMPORARY SUPPORTS. PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER, PRIOR TO ANY REMOVALS.
- <u>ADDITIONAL NOTES:</u> PROVIDE TEMPORARY SHORING TO STRUCTURE ABOVE PRIOR TO ANY
- EXISTING FRAMING SHOWN IS ASSUMED BASED ON SITE REVIEW AND EXISTING DRAWINGS REFERENCED IN NOTES BELOW. CONTRACTOR TO EXPOSE EXISTING STRUCTURE AND REPORT ANY DISCREPANCIES TO THE

- SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: CONCRETE FORMWORK, CONCRETE MIX DESIGN, REINFORCING STEEL, STRUCTURAL STEEL, COLD-FORMED STEEL STUD AND TEMPORARY
- 2. THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN $\frac{1}{4}$ "=1'-0" (1:50). 3. THE STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED, IN WHOLE OR
- IN PART, FOR USE AS SHOP DRAWINGS. 4. EACH DRAWING SUBMITTED FOR CONCRETE FORMWORK. STRUCTURAL STEEL, COLD-FORMED STEEL STUD AND TEMPORARY SHORING SHALL
- BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. 5. CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.

1. SUBMIT CALCULATIONS, BEARING THE SEAL AND SIGNATURE OF PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, FOR STRUCTURAL WORK, IF REQUESTED BY THE CONSULTANT.

1. ALL STRUCTURAL CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.3. ALL CONCRETE CONSTRUCTION SHALL BE

– 25 MPa TYPE N

- IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.1. 2. MINIMUM CONCRETE STRENGTH AT 28 DAYS SHALL BE: – FOOTINGS * – 25 MPa TYPE N – FOUNDATION WALLS * – 25 MPa TYPE F2
- 25 MPa SLUMP SHALL BE $3" \pm 1"$. <u>Snow, ice and rain loads</u> AGGREGATE SHALL BE ¾" MAXIMUM. - IMPORTANCE FACTOR, Is * PROVIDE CRYSTALLINE WATERPROOFING ADMIXTURE AT ELEVATOR PITS. - GROUND SNOW LOAD, S AIR ENTRAINMENT TO BE 6% \pm 1% WHEN EXPOSED TO EXTERIOR. - ASSOCIATED RAIN LOAD. CONTRACTOR TO SUBMIT CONCRETE MIX DESIGN FOR REVIEW - ROOF SNOW LOAD. S.
- 3. THE DEFORMED REINFORCING STEEL SHALL CONFORM TO CSA STANDARD G30.18M-09 GRADE 300R FOR STIRRUPS AND TIES AND GRADE 400R FOR ALL OTHER REINFORCING. UNLESS OTHERWISE NOTED THE REINFORCING LAP LENGTH SHALL BE 'CLASS B' IN SPLICES. ALL REINFORCING HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH A23.1.

- 1½" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE

- 4. WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH CSA G30.5. ALL MESH SHALL BE CHAIRED PRIOR TO THE CONCRETE POUR. LIFTING OF THE MESH DURING THE CONCRETE POUR WILL NOT PERMITTED. ALL SPLICES SHALL BE A
- MINIMUM OF TWO CROSSWIRE SPACINGS PLUS 2". 5. THE REINFORCING COVER FOR CONCRETE SHALL BE: – 3" FOR CONCRETE AGAINST EARTH
- 2" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 20M OR LARGER – 1" FOR INTERIOR CONCRETE. ALL CHAIRS, BOLSTERS, SPACERS AND BAR SUPPORTS SHALL BE IN ACCORDANCE WITH A23.1. 6. FOOTINGS SHALL BEAR ON NATIVE UNDISTURBED SOIL OR ENGINEERED FILL WITH
- 3000 psf (SLS) – 4000 psf (ULS). THE CONTRACTOR SHALL VERIFY THE CAPACITY PRIOR TO PLACEMENT OF CONCRETE.

REINFORCING BAR IS 15M OR SMALLER

A MINIMUM BEARING RESISTANCE OF:

- 7. THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATION OR STEP DOWN FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10. STEP HEIGHT SHALL NOT EXCEED 2'-0".
- 8. KEEP EXCAVATIONS DRY BEFORE CONCRETE IS PLACED. REMOVE ALL LOOSE MATERIAL, SOFT SOIL OR WATER PRIOR TO PLACING CONCRETE. PROVIDE A 3" MUD MAT FOR ALL FOOTINGS BELOW THE WATER TABLE. 9. ALL FOOTINGS SHALL BE CENTRED ON THE WALL UNLESS OTHERWISE NOTED. 10. THE FOOTING DESIGN IS BASED ON INFORMATION AVAILABLE AT THE TIME OF DESIGN. THE FOOTING DESIGN MAY BE ALTERED DURING CONSTRUCTION. IF THE
- 11. PROTECT ALL FOOTINGS, WALLS AND SLABS AGAINST FROST ACTION DURING CONSTRUCTION. ALL EXTERIOR FOOTINGS SHALL FOUNDED BELOW THE FROST LINE, MINIMUM 5'-0" BELOW GRADE.

SITE CONDITIONS WARRANT, BUT ONLY WITH THE EXPRESS PERMISSION OF THE

- 12. DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL THE ELEMENTS PROVIDING LATERAL SUPPORT ARE COMPLETE. PLACE BACKFILL IN A MANNER WHERE THE ELEVATION DIFFERENCE ON EITHER SIDE OF THE WALL IS NO GREATER THAN 1'-6". PROVIDE TEMPORARY SHORING AS REQUIRED. 13. SLAB-ON-GRADE GRADE CONSTRUCTION SHALL BE CAPABLE OF SUPPORTING
- 14. CONSTRUCT CONCRETE WALLS WITHOUT CONTROL JOINTS, UNLESS OTHERWISE NOTED. PROVIDE CHASES AND BEAMS POCKETS IN THE INTERIOR FACE OF THE 15. PROVIDE DOWELS TO WALLS AND COLUMNS TO SUIT THE REINFORCING IN THE

500 lbs/ft2 WITHOUT RELATIVE SETTLEMENT.

WALL OR COLUMN ABOVE.

- 16. ALL ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE HILTI HIT-HY200 (OR APPROVED EQUAL) PROCEDURES.
- 17. PROVIDE DEWATERING AS REQUIRED FOR NEW FOUNDATIONS.

LOADING SUMMARY

- DESIGN STANDARDS ONTARIO BUILDING CODE, 2024, PART 4: STRUCTURAL DESIGN - CAN/CSA-A23.3-19, DESIGN OF CONCRETE STRUCTURES
- CAN/CSA-A23.4-16, DESIGN OF PRECAST CONCRETE STRUCTURES - CAN/CSA-S304-14, MASONRY DESIGN FOR BUILDINGS - CAN/CSA-S16-19. LIMIT STATES DESIGN OF STEEL STRUCTURES - CAN/CSA-S136-16, DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS - CAN/CSA-086-19, ENGINEERING DESIGN IN WOOD

<u>Design Loads</u>

NEW ROOF LOADS:

- MECH. & ELECTRICAL

- 127 COMPOSITE DECK

MISC. MECH. & ELECTRICAL

NEW FLOOR LOADS:

- CEILING

TOTAL

- INSULATION & PROTECTION BOARD

0.16 kPa

0.10 kPa

0.14 kPa

0.20 kPa

1.30 kPa

2.50 kPa

0.35 kPa

0.20 kPa

0.15 kPa

3.45 kPa

ROOFING

- 0.9 (SLS) 1.15 (ULS) 0.9 **kPa (18.80 PS**F 0.4 **kPa (8.35 PSF)** - WIND EXPOSURE FACTOR, CV 1.47 **kPa (30.70 PSF)**
- DRIFT LOADS PER CLAUSE 4.1.6.2.8 SLOPE FACTORS PER CLAUSE 4.1.6.2.(5) TO (7) <u>wind loads</u> APPLIED PER OBC, PART 4, SECTION 4.1.7 IMPORTANCE FACTOR, IW
- REFERENCE VELOCITY PRESSURE FOR STRUCTURAL MEMBERS 0.46 kPa 1/50 YEAR PROBABILITY - REFERENCE VELOCITÝ PRESSURE FOR CLADDING & NON—STRUCTURAĹ MEMBERS 0.36 kPa 1/10 YEAR PROBABILITY – GUST FACTORS Ca: FOR WHOLE & MAIN STRUCTURAL MEMBERS
- FOR SMALL ELEMENTS INCLUDING CLADDING FOR INTERNAL PRESSURES - BUILDING INTERNAL PRESSURE CATEGORY <u>2</u> PER 4.1.7.7

FLOOR LOADS APPLIED PER OBC, PART 4, TABLE 4.1.5.3

- 2.4 kPa (50 PSF 4.8 kPa (100 PSÉ) STAIRS AND EXITS 4.8 kPa (100 PSF) 2.4 kPa `(50 PSF)
- <u>Seismic Sway Bracing</u> PER ARTICLE 4.1.8.18(2) OF THE ONTARIO BUILDING CODE SEISMIC SWAY BRACING DOES NOT APPLY TO NON-STRUCTURAL COMPONENTS AND EQUIPMENT, SHOULD THE BUILDING BE IN SEISMIC CATEGORY SC1 OR SC2. THIS EXEMPTION IS NOT APPLICABLE TO POST—DISASTER BUILDINGS. BUILDING SEISMIC CATEGORY IS SC3, THEREFORE SEISMIC SWAY BRACING OF NON-STRUCTURAL COMPONENTS AND EQUIPMENT IS REQUIRED.

TESTING AND INSPECTIONS: 1. THIRD-PARTY TESTING AND INSPECTIONS SHALL BE PROVIDED IN

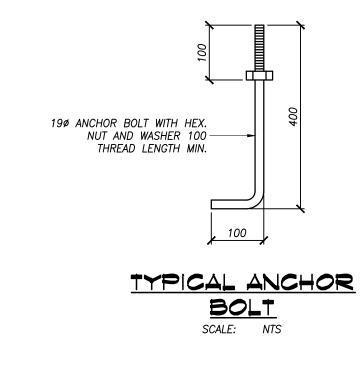
- ACCORDANCE WITH APPLICABLE CODES AND STANDARDS AND AS NOTED BELOW. TESTING AND INSPECTION REQUIREMENTS NOTED IN THE PROJECT SPECIFICATIONS SHALL SUPERSEDE THE FOLLOWING REQUIREMENTS. 2.TESTING OF THE FOOTING SUBGRADE SHALL BE PROVIDED PRIOR TO ALL CONCRETE POURS TO ENSURE CONFORMANCE WITH THE BEARING CAPACITIES NOTED IN THESE PLANS AND IN THE GEOTECHNICAL REPORT. KALOS ENGINEERING INC. SHALL BE NOTIFIED IMMEDIATELY IF EXCAVATIONS ARE REQUIRED TO EXTEND BELOW THE NOTED FOUNDING ELEVATIONS. 3. CONCRETE REINFORCING SHALL BE INSPECTED FOR CONFORMANCE TO THE STRUCTURAL DRAWINGS PRIOR TO EVERY CONCRETE POUR. 4. COMPRESSION TESTING OF CONCRETE SHALL BE IN ACCORDANCE WITH CSA A23.1. A MINIMUM OF ONE STRENGTH TEST SHALL BE COMPLETED FOR TESTS SHALL CONSIST OF ONE SET OF FOUR CYLINDERS.
- EACH 100 m³ OF CONCRETE PLACED AND A MINIMUM OF ONE TEST PER DAY SHALL BE COMPLETED FOR CONCRETE OF A SINGLE MIX DESIGN. 5. THE COMPRESSIVE STRENGTH OF CONCRETE BLOCKS SHALL BE TESTED A MINIMUM OF ONCE PER FLOOR IN ACCORDANCE WITH CSA A165.1. A MINIMUM OF FIVE UNITS OF EACH SIZE OF CONCRETE BLOCK SHALL BE
- 6. MORTAR CUBE COMPRESSIVE STRENGTH SHALL BE TESTED AT LEAST ONCE FOR EACH 500 m² OF MASONRY FOR A PROJECT HAVING MORE THAN 500 m² OF MASONRY AND ONCE FOR EACH 250 m² OF MASONRY FOR A PROJECT HAVING LESS THAN 500 m² OF MASONRY. FOUR SAMPLES SHALL BE TAKEN FOR EACH TEST. 7. GROUT COMPRESSIVE STRENGTH SHALL BE TESTED AT LEAST ONCE FOR EACH 20 m³ OF GROUT FOR A PROJECT HAVING MORE THAN 20 m³ OF GROUT AND ONCE FOR EACH 10 m³OF GROUT FOR A PROJECT HAVING LESS THAN 20 m³ OF GROUT. FOUR SAMPLES SHALL BE TAKEN FOR EACH
- 8. STRUCTURAL STEEL INSPECTIONS AND TESTING SHALL BE AS FOLLOWS: a. INSPECTIONS SHALL BE PERFORMED BY A FIRM CERTIFIED TO CSA W178.1, EXCEPT THAT VISUAL INSPECTION MAY ALSO BE PERFORMED BY PERSONS CERTIFIED TO LEVEL 2 OR 3 OF CSA W178.2. b. INSPECTORS SHALL VERIFY THAT THE ERECTORS ARE CWB CERTIFIED.
- c. CONNECTIONS SHALL BE INSPECTED FOR CONFORMANCE TO THE ENGINEERED DESIGN DETAILS. d. BOLTED CONNECTIONS SHALL BE INSPECTED IN ACCORDANCE WITH CSA S16. VERIFICATION OF CONNECTION FIT-UP AND BOLT TENSION SHALL BE e. WELDED CONNECTIONS SHALL BE INSPECTED IN ACCORANCE WITH CSA S16 AND CSA W59. VISUAL INSPECTIONS SHALL BE PROVIDED TO ENSURE PROPER CONNECTION FIT—UP AND REVIEW OF COMPLETED WELDS. f. INSPECTION FREQUENCY SHALL BE A MINIMUM OF ONCE PER FLOOR OR

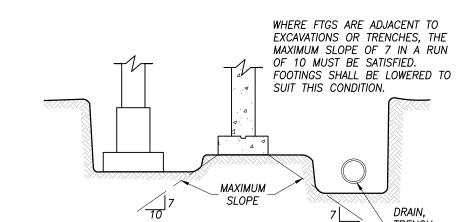
9. METAL DECK THICKNESS, COATING GRADE, SPOT WELDS, MECHANICAL

FASTENERS, AND CRIMPING SHALL BE INSPECTED BY A CERTIFIED

ONCE PER ERECTION PHASE.

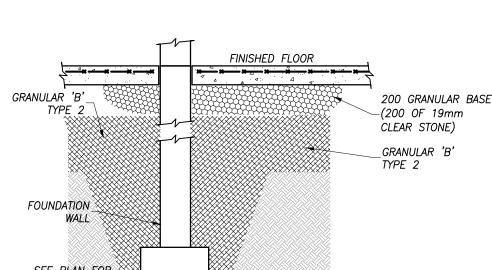
INSPECTOR AT LEAST ONCE PER LEVEL.





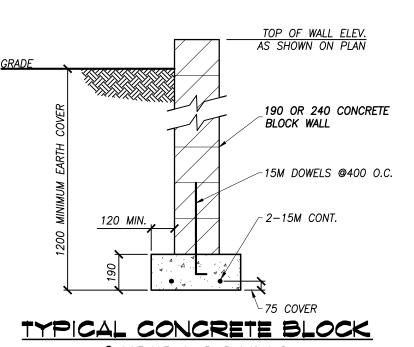
ELEVATIONS OF ADJACENT FOOTINGS AND EXCAYATIONS

SCALE: NTS

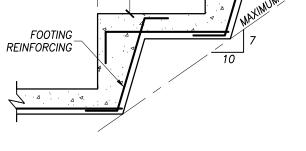


U/S FOOTING ELEV. (REQUIRED BOTH SIDES OF ALL INTERIOR AND EXTERIOR FOUNDATION WALLS) **NOTE:** NATIVE MATERIAL IS UNSUITABLE FOR BACKFILL TYPICAL EXCAVATION AND BACKFILL AT FOUNDATION WALLS

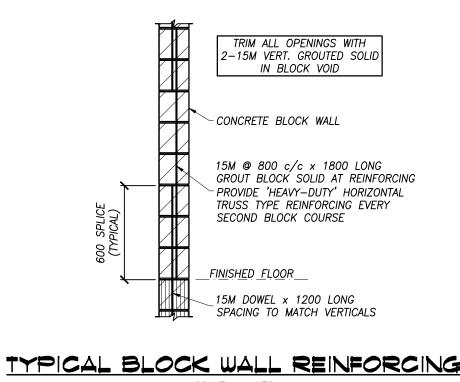
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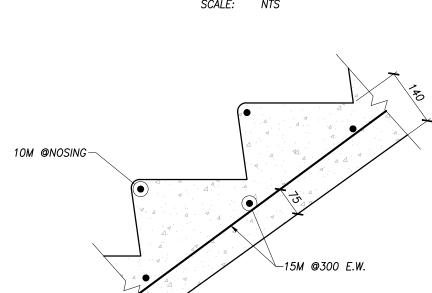


SCALE: NTS

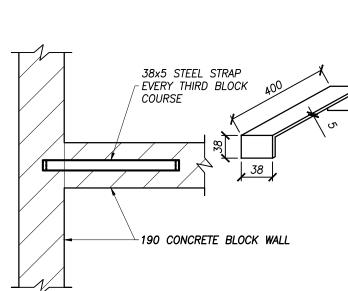


15M @300 O.C CONCRETE SLAB -INTO EXISTING SLAB, MIN 150 ON GRADE **EMBEDMENT** TYPICAL STEPPING OF TYPICAL SLAB REPAIR DETAIL WALL FOOTING DETAIL

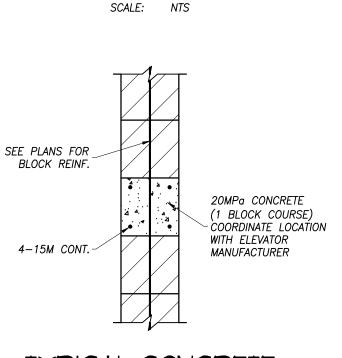


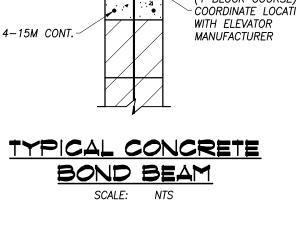


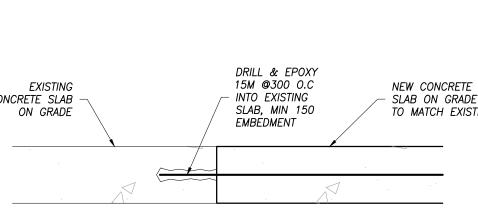
TYPICAL CAST-IN-PLACE STAIR REINFORCING SCALE: NTS

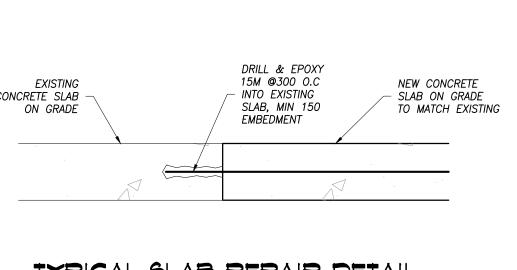


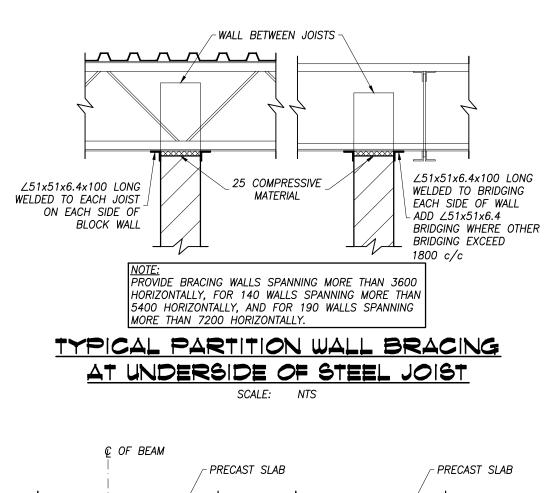
TYPICAL INTERSECTION OF CONCRETE BLOCK WALLS

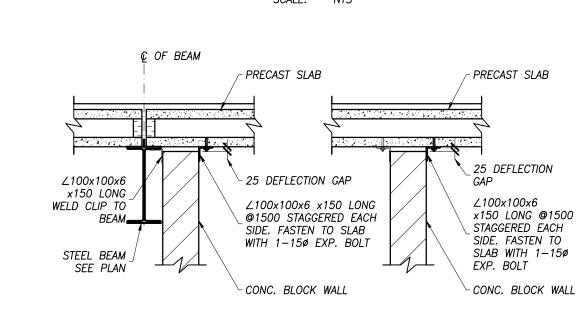


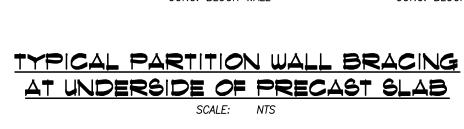


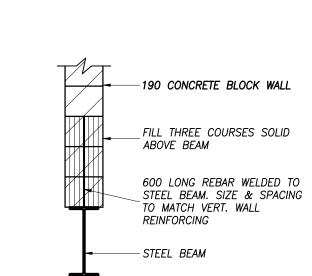




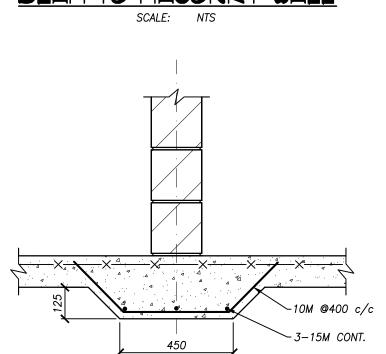


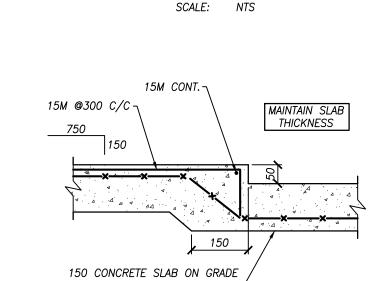










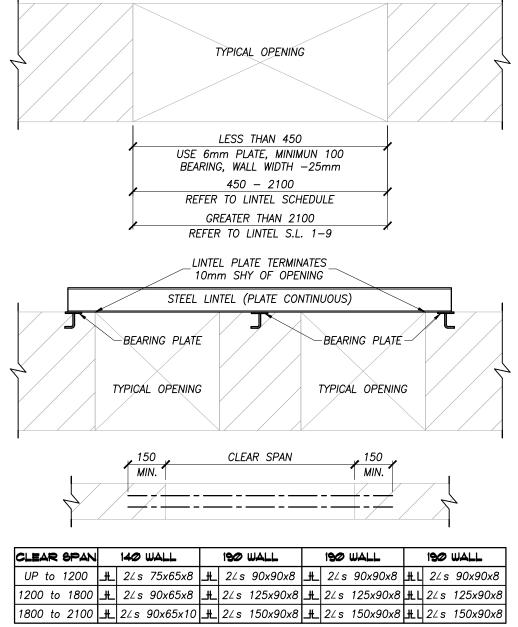


w/ 150x150x¹⁵⁰/₁₅₀ W.W.F.⁻

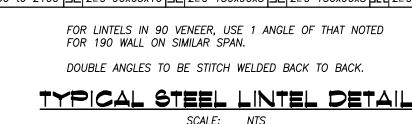
TYPICAL THICKENING OF SLAB ON

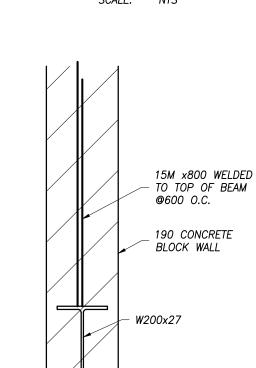
GRADE UNDER PARTITION WALLS

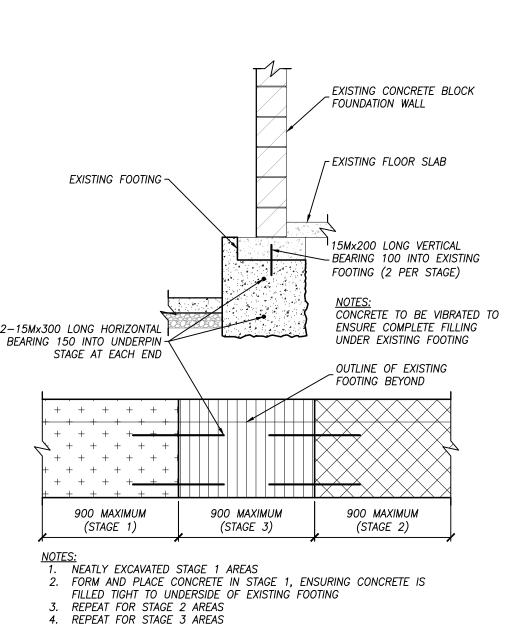
TYPICAL DETAIL AT FLOOR DEPRESSION



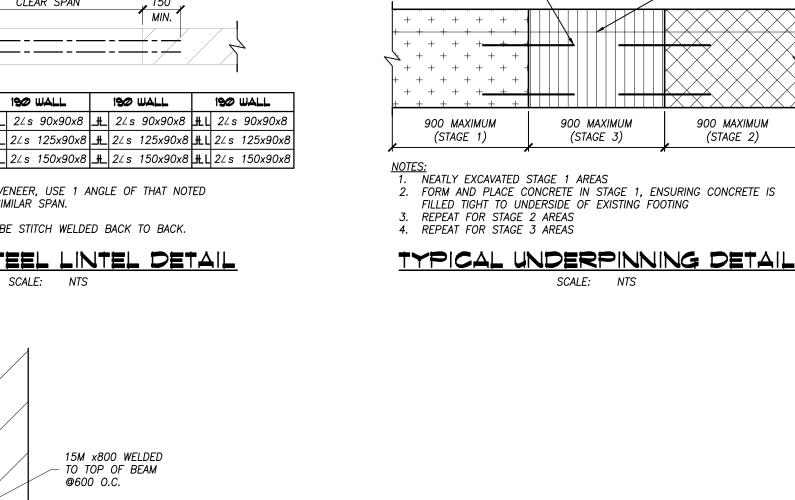
STEEL LINTEL



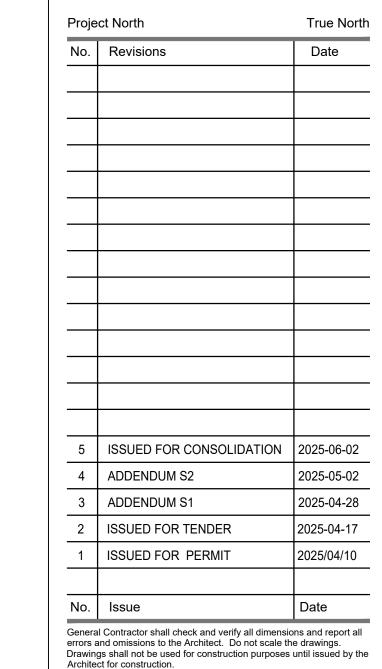




SCALE: NTS







Halton District School Board

2050 Guelph Line

Burlington, Ontario

T. A. BLAKELOCK H.S.

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Tel: 905-333-9119

Mechanical and Electrical Consultants

EXP

1266 S. Service Rd,

Stoney Creek, Ontario, L8E 5R9

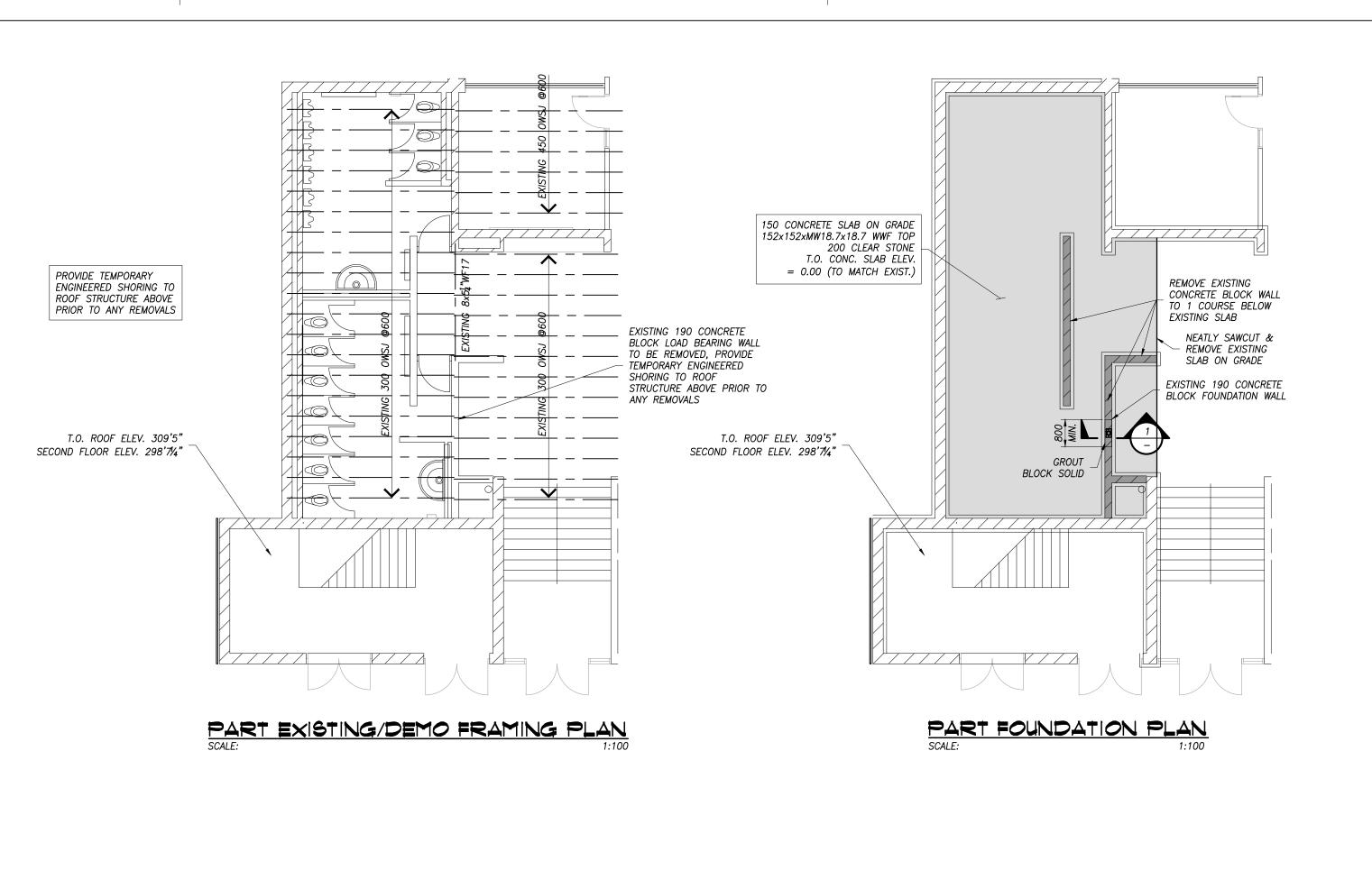
Tel: 905-525-6069

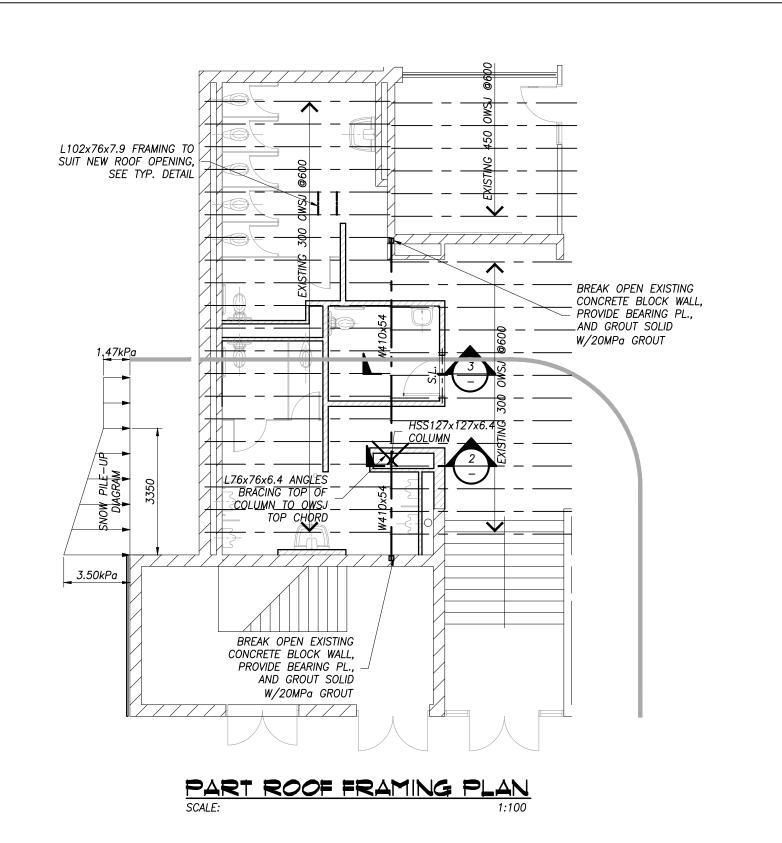
Kalos Engineering Inc

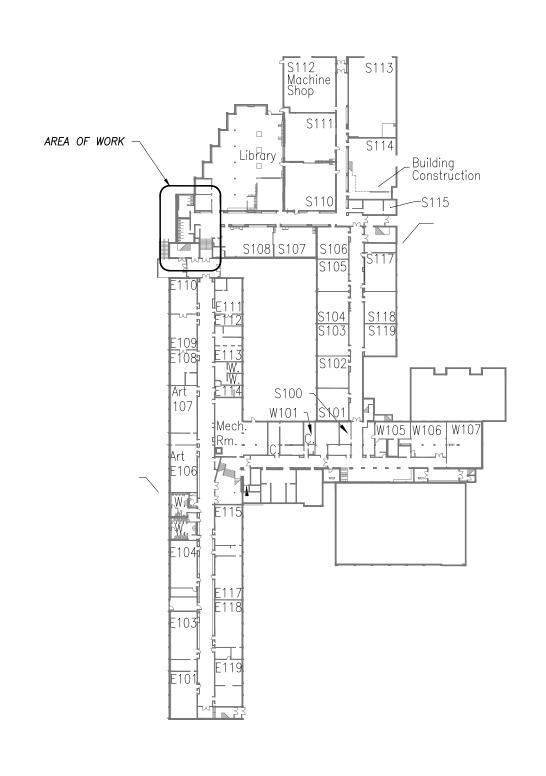
www.snyderarchitects.ca

TYPICAL **NOTES**

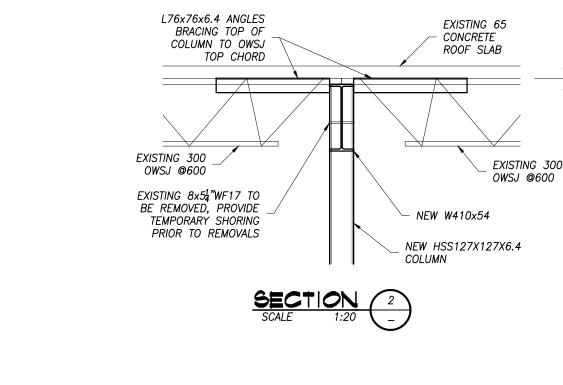
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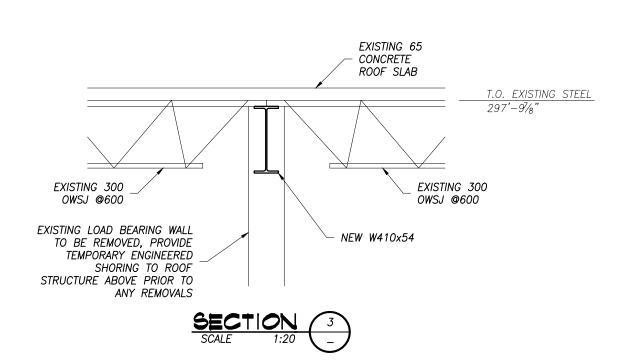


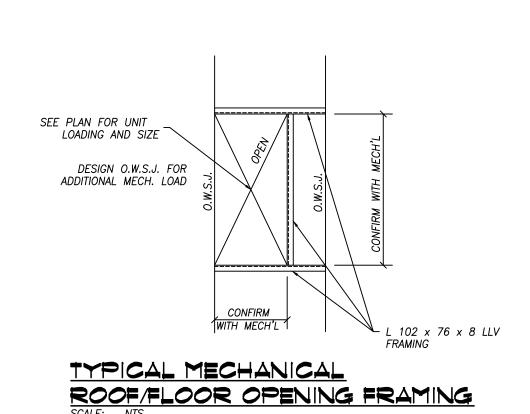




KEY PLAN - GROUND FLOOR FRAMING







NEW HSS127X127X6.4

_ CONCRETE BLOCK WALL

TO 1 COURSE BELOW EXISTING SLAB

EXISTING FOOTING C/W

COLUMN

REMOVE EXISTING

EXISTING LOAD BEARING WALL TO BE REMOVED, PROVIDE

STRUCTURE ABOVE PRIOR TO

150x300x12 BASE PL.

C/W 2-19ø ANCHORS

GROUTED INTO EXISTING

CONCRETE BLOCK WALL, GROUT WALL SOLID FULL
HEIGHT W/20MPa GROUT

TEMPORARY ENGINEERED

EXISTING 100

CONCRETE SLAB -

ON GRADE

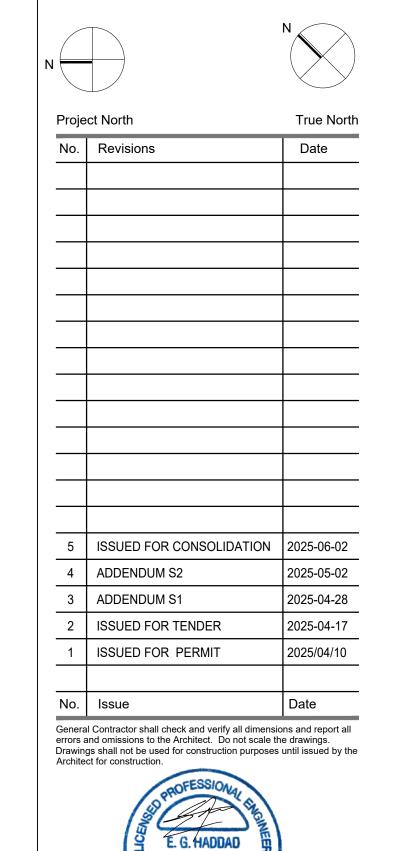
SHORING TO ROOF

ANY REMOVALS

SHORING NOTES

- 1. ALL FRAMES AND SHORING JACKS TO BE PLUMB AND LEVEL.
- 2. SHORING JACKS TO BE DESIGNED BY THE SUPPLIER FOR THE LOADS AND HEIGHTS SHOWN, INCLUDING BRACING.

 3. MAY EXTENSION OF SCREWIACKS WILL BE 4000000 UNITED.
- 3. MAX. EXTENSION OF SCREWJACKS WILL BE 400mm UNLESS NOTED.4. SCAFFOLDING SHALL BE ERECTED IN ACCORDANCE TO C.S.A. CODE
- S269.1.
 5. SHORING TO REMAIN IN PLACE UNTIL BEAM AND ALL BRACING IS COMPLETELY INSTALLED.
- 6. CONTRACTOR TO PREPARE AND SUBMIT FULL SHORING DRAWINGS FOR APPROVAL FOR ALL TEMPORARY SUPPORTS. PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER, PRIOR TO ANY REMOVALS.
- ADDITIONAL NOTES:
- PROVIDE TEMPORARY SHORING TO STRUCTURE ABOVE PRIOR TO ANY REMOVALS.
 EXISTING FRAMING SHOWN IS ASSUMED BASED ON SITE REVIEW AND EXISTING DRAWINGS REFERENCED IN NOTES BELOW. CONTRACTOR TO EXPOSE EXISTING STRUCTURE AND REPORT ANY DISCREPANCIES TO THE



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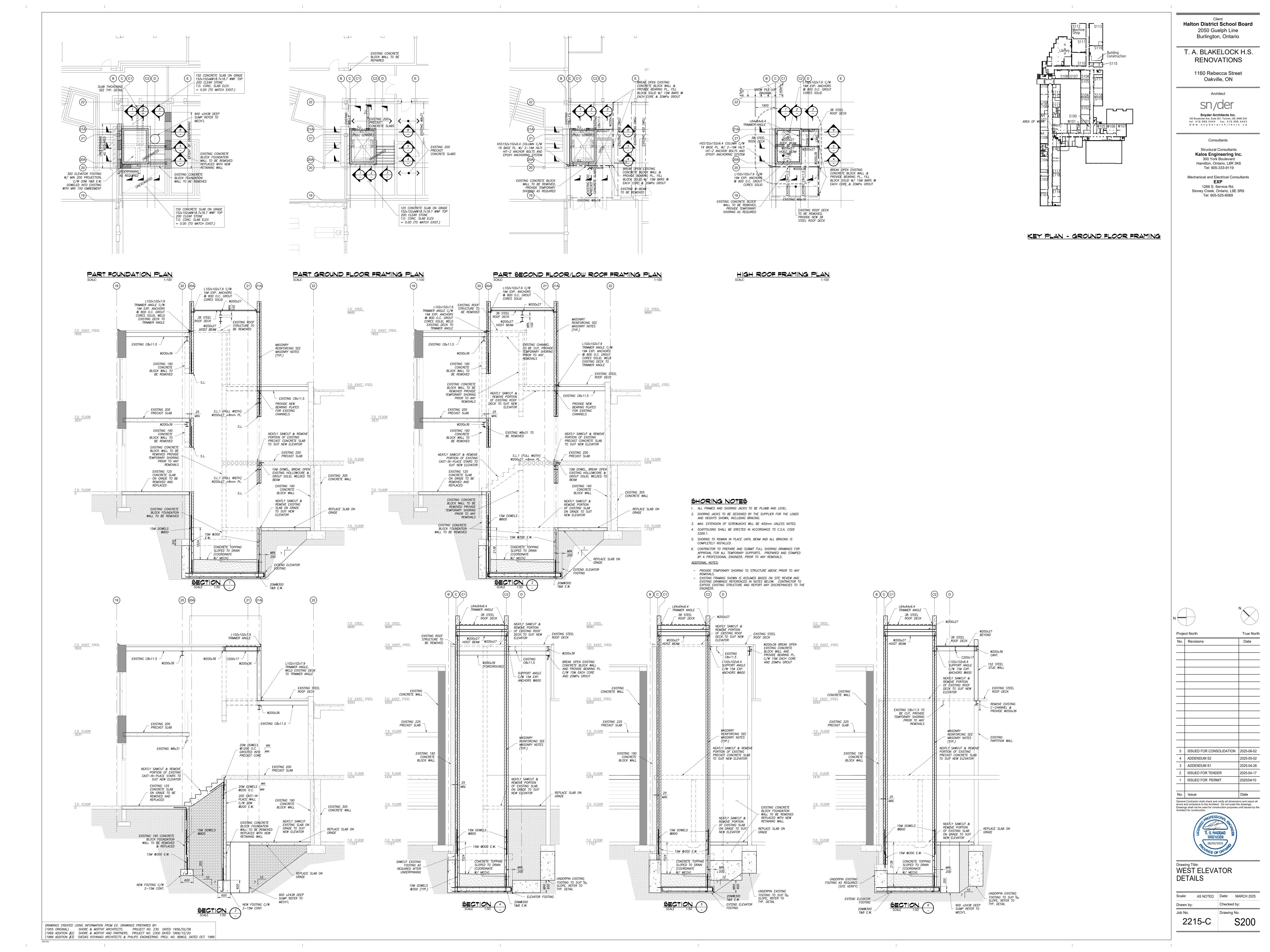
Mechanical and Electrical Consultants **EXP**1266 S. Service Rd,

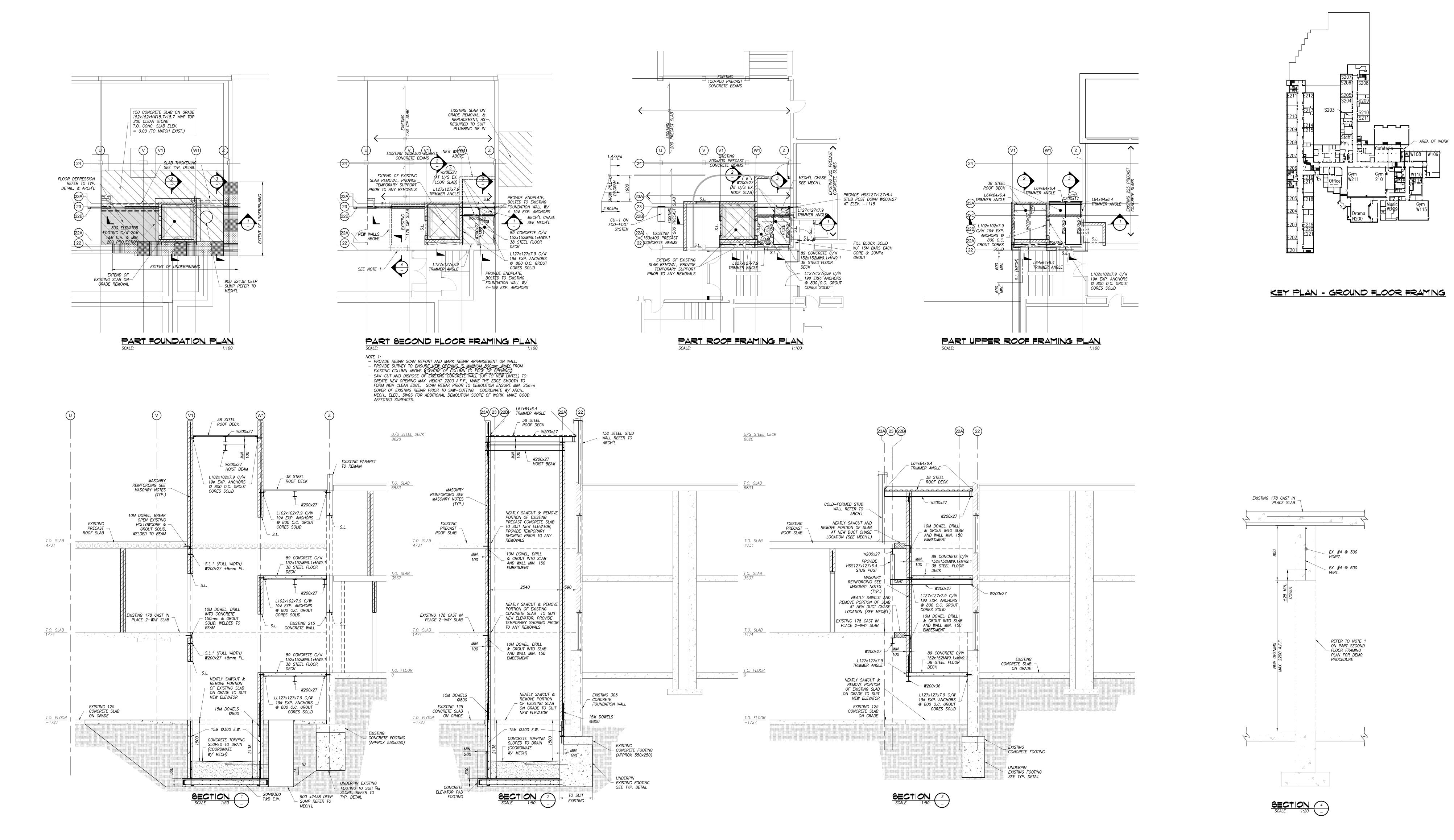
Stoney Creek, Ontario, L8E 5R9

Tel: 905-525-6069

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Proje	ect North	True
No.	Revisions	Date
5	ISSUED FOR CONSOLIDATION	2025-0
4	ADDENDUM S2	2025-0
3	ADDENDUM S1	2025-0
2	ISSUED FOR TENDER	2025-04
1	ISSUED FOR PERMIT	2025/04
No.	Issue	Date

Halton District School Board 2050 Guelph Line Burlington, Ontario

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w w w . s n y d e r a r c h i t e c t s . c a

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Mechanical and Electrical Consultants **EXP**1266 S. Service Rd,

Stoney Creek, Ontario, L8E 5R9

Tel: 905-525-6069

Drawing Title:
EAST ELEVATOR
DETAILS

Scale: AS NOTED Date: MARCH 2025

Drawn by: Checked by:

Job No. Drawing No.

2215-C S300