### GENERAL NOTES 1. CHECK ALL DIMENSIONS ON THESE DRAWINGS WITH ALL OTHER DRAWINGS,

- INCLUDING BUT NOT LIMITED TO DRAWINGS PREPARED ARCHITECTURAL. MECHANICAL OR ELECTRICAL CONSULTANTS. REPORT ANY INCONSISTENCIES TO THE ARCHITECT OR ENGINEER PRIOR TO COMMENCING WITH THE WORK. DO NOT SCALE THE DRAWINGS.
- 2. THE DESIGN LIVE LOADS ARE INDICATED ON THE DRAWINGS. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LOADS. 3. THE COMPLETED STRUCTURE IS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING AND ANY

OTHER TEMPORARY OR PERMANENT MEASURES AS REQUIRED DURING

SUPPORT OF EXISTING OR ADJACENT STRUCTURES AS REQUIRED. ALL

BRACING AND SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY

- 4. CONSTRUCTION FEATURES NOT FULLY SHOWN ARE COMPARABLE TO SIMILAR CONDITION DETAILS. 5. REFER TO OTHER CONSULTANTS DRAWINGS FOR DETAILS OF OPENINGS,
- PITS, CHAMFERS, DEPRESSIONS NOT INDICATED ON THE STRUCTURAL 6. ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE LATEST ONTARIO
- BUILDING CODE, LATEST APPLICABLE REGULATIONS, AND GOOD CONSTRUCTION PRACTICES.
- 7. THE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS. 8. CLARIFY ANY QUERIES WITH THE ENGINEER REGARDING THE INTERPRETATION OF THE DRAWINGS, PRIOR TO THE COMMENCEMENT OF ANY WORK.

## MASONRY NOTES

- 1. ALL STRUCTURAL ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD S304.1. ALL MASONRY CONSTRUCTION SHALL BE IN 10. ALL STEEL EXPOSED TO THE EXTERIOR TO BE HOT DIP GALVANIZED. ACCORDANCE WITH CSA STANDARD A371. ALL MASONRY CONNECTORS, REINFORCING AND TYING SHALL BE IN ACCORDANCE WITH CSA A370. ALL 11. ERECT STRUCTURAL STEEL IN ACCORDANCE WITH CSA S16 AND IN MORTAR AND GROUT SHALL BE IN ACCORDANCE WITH A179.
- 2. ALL CONCRETE BLOCKS SHALL BE NORMAL WEIGHT TYPE H/15/A/M UNLESS OTHERWISE NOTED. MORTAR SHALL BE TYPE S FOR LOADBEARING AND TYPE N FOR NON-LOADBEARING. 3. VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 6000mm. REFER TO ARCHITECTURAL DRAWING FOR DETAILS AND
- 4. TRIM ALL OPENINGS WITH 2-15M BARS. 5. GROUT SHALL CONSIST OF ON ONE PART PORTLAND CEMENT, THREE PARTS SAND (MAXIMUM AGGREGATE SIZE SHALL BE 10mm) WITH WATER TO PROVIDE A MINIMUM 10MPa COMPRESSIVE STRENGTH AT 28 DAYS.
- SLUMP SHALL BE 200mm TO 250mm. 6. ALL CELLS CONTAINING REINFORCING SHALL BE GROUTED SOLID. TWO BLOCK COURSES BELOW BEARING PLATES SHALL BE GROUTED SOLID.
- 7. THE MASONRY SHALL BE CONSTRUCTED EVENLY WITH MAXIMUM LIFTS OF 1200 PER DAY. DO NOT TOOTH AND BOND OR STACK BOND MASONRY. RAKE BACK ENDS OF UNFINISHED WALLS. 8. ALL MORTAR JOINTS SHALL BE TOOLED (CONCAVE). A MINIMUM BED JOINT
- OF 6mm IS REQUIRED FOR THE STARTING COURSE TO A MAXIMUM OF 20mm. THE BED JOINTS SHALL BE 10mm.
- 9. PROVIDE VERTICAL AND HORIZONTAL REINFORCING AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS. - 140 CONCRETE BLOCK - HEAVY DUTY TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE.

- 190 CONCRETE BLOCK - 15M VERTICAL AT 800 O.C. & HEAVY DUTY

- TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE. 10. THE HORIZONTAL REINFORCING AT EXTERIOR WALLS SHALL BE GALVANIZED. DO NOT EXTEND HORIZONTAL REINFORCING THROUGH CONTROL JOINTS UNLESS OTHERWISE NOTED.
- 11. PROVIDE A STEEL LINTEL OVER ALL OPENINGS OR RECESSES INCLUDING OPENINGS FOR MECHANICAL AND ELECTRICAL COMPONENTS. ALL EXTERIOR LINTELS TO BE HOT DIP GALVANIZED.
- 12. BUILD THE MASONRY SOLID AROUND ALL BEAM, LINTEL AND JOIST POCKETS. INSTALL BEARING PLATES AT THE SPECIFIED ELEVATION AND GROUT THE PLATE INTO THE WALL A MINIMUM OF 400mm. 13. PROVIDE TEMPORARY BRACING AS REQUIRED TO SUPPORT THE MASONRY

WALLS IN CONSTRUCTION. PROTECT THE MASONRY WALLS FROM THE ELEMENTS AT ALL TIMES EXCEPT DURING CONSTRUCTION PROGRESS.

- STRUCTURAL STEEL NOTES 1. ALL STRUCTURAL STEEL ELEMENTS, INCLUDING DESIGN OF ELEMENTS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH CAN/CSA S16. ALL STRUCTURAL STEEL SHALL CONFORM TO CSA G40.21 (300W) EXCEPT W SECTIONS AND PLATES G40.21 (350W), HSS MEMBERS G40.21 (350W)
- 2. THE DESIGN WIND LOADING IS 1.2kN/m² (25 PSF) DETERMINED BY CLASS C OR ASTM A500 GRADE C, ANCHOR BOLTS ASTM A307, COLD O.B.C. REQUIREMENTS AND CAN-S136. DEFLECTION IS LIMITED TO FORMED SECTIONS ASTM A570M GRADE 350W. UNLESS OTHERWISE NOTED, ALL SECTIONS SHALL BE PRIME PAINTED WITH THE SURFACE PREPARATION AND PAINTING PROCEDURES IN ACCORDANCE WITH 3. THE DESIGN OF FRAMING SYSTEM IS BASED ON PUBLISHED STUD SECTION PROPERTIES BY BAILEY METAL PRODUCTS LIMITED. CAN/CGSB 85.10. ALL FIRE RATED STEEL TO BE LEFT UNPRIMED. MATERIAL
- 3. ALL WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH CAN/CSA W59. THE STEEL FABRICATOR SHALL BE FULLY QUALIFIED UNDER THE REQUIREMENTS BY THE CANADIAN WELDING BUREAU IN CONFORMANCE WITH CAN/CSA W47.1.
- 4. DESIGN ALL MOMENT AND SHEAR CONNECTIONS FOR THE FULL CAPACITY OF THE SMALLER MEMBER IN THE CONNECTION UNLESS OTHERWISE
- 3. GALVANIZING TO BE HOT-DIP PROCESS, G90 (Z275). 5. PROVIDE MINIMUM BEARING LENGTH OF STEEL MEMBERS AS FOLLOWS: – ON MASONRY – 150mm 1. METHOD OF CONSTRUCTION SHALL BE BY STICK BUILDING ON SITE.
- ON STEEL 90mm 6. THE BASE PLATE AND BEARING PLATE GROUT SHALL BE OF THE CEMENTITIOUS NON-SHRINK TYPE.

CONFORMANCE WITH THE APPROVED SHOP DRAWINGS.

ACCORDANCE WITH CSA S16.

- 2. CONNECTIONS SHALL BE ACCOMPLISHED BY SELF DRILLING SCREWS AND OTHER FASTENERS AS SHOWN ON THESE DRAWINGS. PENETRATION 7. FULLY WELD THE BASE PLATE TO THE COLUMN TO DEVELOP THE ANCHOR BOLTS. PROVIDE CAP PLATES ON ALL COLUMNS. PROVIDE 6mm CAP PLATES ON ALL COLUMNS.
- BEYOND JOINED MATERIALS SHALL BE NOT LESS THAN THREE EXPOSED THREADS. ALL CONNECTORS USED IN ASSEMBLIES SHALL BE OF CORROSION RESISTANT MATERIAL COMPATIBLE WITH GALVANIZED COATINGS WITH A MINIMUM COATING THICKNESS OF 0.039mm ZINC OF CADMIUM PLATES. NO BLACK CONNECTORS WILL BE ACCEPTED. 8. PROVIDE MINIMUM 175x10x175 BEARING PLATES FOR ALL STRUCTURAL SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER. STEEL c/w 2-150 ANCHORS UNLESS OTHERWISE NOTED.
- SCREWS COVERED BY SHEATHING MATERIALS SHALL HAVE LOW PROFILE 9. ALL BOLTS SHALL BE TIGHTENED WITH A SUITABLE TORQUE WRENCH IN

EXECUTION

GENERAL

EXTERIOR WALL SYSTEM ONLY.

4. WIRE TYING IS NOT PERMITTED IN STRUCTURAL APPLICATIONS. 5. CUTTING OF STEEL FRAMING MEMBERS SHALL BE BY SAW OR SHEAR. NO TORCH OR MANUAL CUTTING IS PERMITTED.

LIGHT GAUGE STEEL FRAMING NOTES

1. THESE NOTES APPLY TO THE STEEL STUD FRAMING COMPONENT OF THE

1. THE MINIMUM BASE METAL THICKNESS FOR ALL METAL WALL

(340) CLASS 1 FOR 1.52mm MATERIAL AND THICKER.

COMPONENTS, EXCLUDING COATINGS ARE NOTED ON THE DRAWINGS.

2. STEEL MEETS THE REQUIREMENTS OF A.S.T.M. A653/A653M SS GRADE

33 (230) FOR 1.22mm MATERIAL AND THINNER, AND SS GRADE 50

- 6. SPLICING OF STUDS OR TRACK IS NOT PERMITTED EXCEPT AS NOTED 7. BRIDGING SHALL BE OF SIZE, SPACING AND TYPE SHOWN ON THE
- DRAWINGS AND SHALL BE INSTALLED SO AS TO PROVIDE RESISTANCE TO MINOR AXIS BENDING AND ROTATION OF STUDS. PROVIDE BRIDGING AT 1200mm c/c MAXIMUM.
- 8. TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED. 9. STUDS SHALL SEAT INTO TOP AND BOTTOM TRACKS WITH THE GAP BETWEEN THE END OF THE STUD AND WEB OF THE TRACK NOT TO
- 10. VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/1000
- 11. HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/1000 OF THEIR RESPECTIVE LENGTHS.
- 12. SPACING OF STUDS SHALL BE WITHIN 3mm FROM DESIGN SPACING PROVIDED THAT CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF THE FINISHING MATERIALS.

## SHORING NOTES

- 1. ALL FRAMES AND SHORING JACKS TO BE PLUMB AND LEVEL. 2. SHORING JACKS TO BE DESIGNED BY THE SUPPLIER FOR THE LOADS AND HEIGHTS SHOWN, INCLUDING BRACING.
- 3. MAX. EXTENSION OF SCREWJACKS WILL BE 400mm UNLESS NOTED. 4. SCAFFOLDING SHALL BE ERECTED IN ACCORDANCE TO C.S.A. CODE
- 5. SHORING TO REMAIN IN PLACE UNTIL BEAM AND ALL BRACING IS COMPLETELY INSTALLED. 6. CONTRACTOR TO PREPARE AND SUBMIT FULL SHORING DRAWINGS FOR
- APPROVAL FOR ALL TEMPORARY SUPPORTS. PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER, PRIOR TO ANY REMOVALS. <u>ADDITIONAL NOTES:</u>
- PROVIDE TEMPORARY SHORING TO STRUCTURE ABOVE PRIOR TO ANY EXISTING FRAMING SHOWN IS ASSUMED BASED ON SITE REVIEW AND EXISTING DRAWINGS REFERENCED IN NOTES BELOW. CONTRACTOR TO EXPOSE EXISTING STRUCTURE AND REPORT ANY DISCREPANCIES TO THE

- . SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: CONCRETE FORMWORK, CONCRETE MIX DESIGN, REINFORCING STEEL, STRUCTURAL STEEL, COLD-FORMED STEEL STUD AND TEMPORARY
- 2. THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN  $\frac{1}{4}$ "=1'-0" (1:50).
- 3. THE STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED, IN WHOLE OR IN PART, FOR USE AS SHOP DRAWINGS. 4. EACH DRAWING SUBMITTED FOR CONCRETE FORMWORK, STRUCTURAL STEEL. COLD—FORMED STEEL STUD AND TEMPORARY SHORING SHALL
- BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. 5. CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.

PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, FOR STRUCTURAL WORK, IF REQUESTED BY THE CONSULTANT.

1. SUBMIT CALCULATIONS, BEARING THE SEAL AND SIGNATURE OF

# CONCRETE NOTES

- 1. ALL STRUCTURAL CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.3. ALL CONCRETE CONSTRUCTION SHALL BE
- IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.1. 2. MINIMUM CONCRETE STRENGTH AT 28 DAYS SHALL BE: – FOOTINGS \* – 25 MPa TYPE N
- FOUNDATION WALLS \* 25 MPa TYPE F2 SLAB ON GRADE – 25 MPa TYPE N TOPPING – 25 MPa SLUMP SHALL BE  $3" \pm 1"$ . AGGREGATE SHALL BE  $\frac{3}{4}$ " MAXIMUM.
- \* PROVIDE CRYSTALLINE WATERPROOFING ADMIXTURE AT ELEVATOR PITS. AIR ENTRAINMENT TO BE 6%  $\pm$  1% WHEN EXPOSED TO EXTERIOR. CONTRACTOR TO SUBMIT CONCRETE MIX DESIGN FOR REVIEW
- 3. THE DEFORMED REINFORCING STEEL SHALL CONFORM TO CSA STANDARD G30.18M-09 GRADE 300R FOR STIRRUPS AND TIES AND GRADE 400R FOR ALL OTHER REINFORCING. UNLESS OTHERWISE NOTED THE REINFORCING LAP LENGTH SHALL BE 'CLASS B' IN SPLICES. ALL REINFORCING HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH A23.1.
- 4. WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH CSA G30.5. ALL MESH SHALL BE CHAIRED PRIOR TO THE CONCRETE POUR. LIFTING OF THE MESH DURING THE CONCRETE POUR WILL NOT PERMITTED. ALL SPLICES SHALL BE A

### MINIMUM OF TWO CROSSWIRE SPACINGS PLUS 2". 5. THE REINFORCING COVER FOR CONCRETE SHALL BE:

- 3" FOR CONCRETE AGAINST EARTH - 1½" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 15M OR SMALLER - 2" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 20M OR LARGER – 1" FOR INTERIOR CONCRETE. ALL CHAIRS, BOLSTERS, SPACERS AND BAR
- SUPPORTS SHALL BE IN ACCORDANCE WITH A23.1. 6. FOOTINGS SHALL BEAR ON NATIVE UNDISTURBED SOIL OR ENGINEERED FILL WITH A MINIMUM BEARING RESISTANCE OF: – 3000 psf (SLS)
- THE CONTRACTOR SHALL VERIFY THE CAPACITY PRIOR TO PLACEMENT OF CONCRETE.

LINE, MINIMUM 5'-0" BELOW GRADE.

– 4000 psf (ULS).

- 7. THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATION OR STEP DOWN FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10. STEP HEIGHT SHALL NOT EXCEED 2'-0"
- 8. KEEP EXCAVATIONS DRY BEFORE CONCRETE IS PLACED. REMOVE ALL LOOSE MATERIAL, SOFT SOIL OR WATER PRIOR TO PLACING CONCRETE. PROVIDE A 3" MUD MAT FOR ALL FOOTINGS BELOW THE WATER TABLE. 9. ALL FOOTINGS SHALL BE CENTRED ON THE WALL UNLESS OTHERWISE NOTED.
- SITE CONDITIONS WARRANT, BUT ONLY WITH THE EXPRESS PERMISSION OF THE 11. PROTECT ALL FOOTINGS, WALLS AND SLABS AGAINST FROST ACTION DURING CONSTRUCTION. ALL EXTERIOR FOOTINGS SHALL FOUNDED BELOW THE FROST

10. THE FOOTING DESIGN IS BASED ON INFORMATION AVAILABLE AT THE TIME OF DESIGN. THE FOOTING DESIGN MAY BE ALTERED DURING CONSTRUCTION. IF THE

- 12. DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL THE ELEMENTS PROVIDING LATERAL SUPPORT ARE COMPLETE. PLACE BACKFILL IN A MANNER WHERE THE ELEVATION DIFFERENCE ON EITHER SIDE OF THE WALL IS NO GREATER THAN 1'-6". PROVIDE TEMPORARY SHORING AS REQUIRED.
- 13. SLAB-ON-GRADE GRADE CONSTRUCTION SHALL BE CAPABLE OF SUPPORTING 500 lbs/ft2 WITHOUT RELATIVE SETTLEMENT. 14. CONSTRUCT CONCRETE WALLS WITHOUT CONTROL JOINTS, UNLESS OTHERWISE NOTED. PROVIDE CHASES AND BEAMS POCKETS IN THE INTERIOR FACE OF THE
- 15. PROVIDE DOWELS TO WALLS AND COLUMNS TO SUIT THE REINFORCING IN THE WALL OR COLUMN ABOVE. 16. ALL ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE HILTI
- 17. PROVIDE DEWATERING AS REQUIRED FOR NEW FOUNDATIONS.

HIT-HY200 (OR APPROVED EQUAL) PROCEDURES.

## LOADING SUMMARY

- <u>Design Standards</u> ONTARIO BUILDING CODE, 2024, PART 4: STRUCTURAL DESIGN - CAN/CSA-A23.3-19, DESIGN OF CONCRETE STRUCTURES
- CAN/CSA-A23.4-16, DESIGN OF PRECAST CONCRETE STRUCTURES - CAN/CSA-S304-14, MASONRY DESIGN FOR BUILDINGS - CAN/CSA-S16-19, LIMIT STATES DESIGN OF STEEL STRUCTURES - CAN/CSA-S136-16, DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS - CAN/CSA-086-19, ENGINEERING DESIGN IN WOOD
- SNOW, ICE AND RAIN LOADS - IMPORTANCE FACTOR, Is 0.9 **(SLS)** 1.15 **(ULS)** - GROUND SNOW LOAD, S 0.9 **kPa (18.80 PSF** - ASSOCIATED RAIN LOAD. 0.4 **kPa (8.35 PSF)** - WIND EXPOSURE FACTOR, Cw.
- ROOF SNOW LOAD, S, 1.47 **kPa (30.70 PSF)** - DRIFT LOADS PER CLAUSE 4.1.6.2.8 SLOPE FACTORS PER CLAUSE 4.1.6.2.(5) TO (7)
- APPLIED PER OBC, PART 4, SECTION 4.1.7 IMPORTANCE FACTOR, IW - REFERENCE VELOCITY PRESSURE FOR STRUCTURAL MEMBÉRS 0.46 kPa 1/50 YEAR PROBABILITY - REFERENCE VELOCITÝ PRESSURE FOR CLADDING & NON—STRUCTURAĹ MEMBERS 0.36 kPa 1/10 YEAR PROBABILITY (7.52 PSF)
- GUST FACTORS Cq: FOR WHOLE & MAIN STRUCTURAL MEMBERS FOR SMALL ELEMENTS INCLUDING CLADDING FOR INTERNAL PRESSURES

## - BUILDING INTERNAL PRESSURE CATEGORY \_2\_ PER 4.1.7.7 FLOOR LOADS

APPLIED PER OBC, PART 4, TABLE 4.1.5.3 2.4 kPa (50 PSF) CORRIDORS

wind loads

4.8 kPa (100 PSÉ) STAIRS AND EXITS 4.8 kPa (100 PSF) 2.4 kPa `(50 PSF)

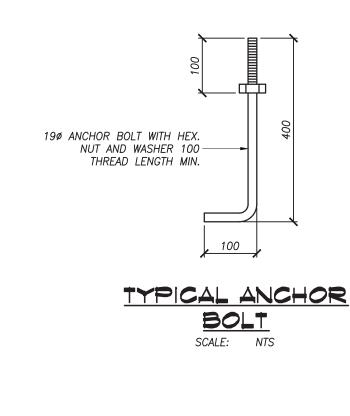
<u>Seismic Sway Bracing</u> PER ARTICLE 4.1.8.18(2) OF THE ONTARIO BUILDING CODE SEISMIC SWAY BRACING DOES NOT APPLY TO NON-STRUCTURAL COMPONENTS AND EQUIPMENT. SHOULD THE BUILDING BE IN SEISMIC CATEGORY SC1 OR SC2. THIS EXEMPTION IS NOT APPLICABLE TO POST—DISASTER BUILDINGS. BUILDING SEISMIC CATEGORY IS SC3, THEREFORE SEISMIC SWAY BRACING OF

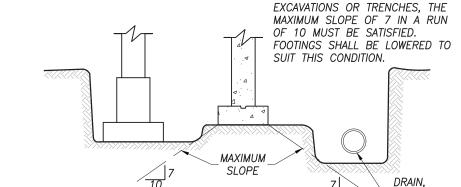
NON-STRUCTURAL COMPONENTS AND EQUIPMENT IS REQUIRED.

DESIGN LOADS NEW ROOF LOADS: ROOFING - INSULATION & PROTECTION BOARD 0.16 kPa 0.10 kPa 0.14 kPa - MECH. & ELECTRICAL 0.35 kPa 0.20 kPa CEILING 1.30 kPa NEW FLOOR LOADS: - 127 COMPOSITE DECK 2.50 kPa MISC. MECH. & ELECTRICAL 0.35 kPa 0.25 kPa CEILING 0.20 kPa FRAMING 0.15 kPa

3.45 kPa

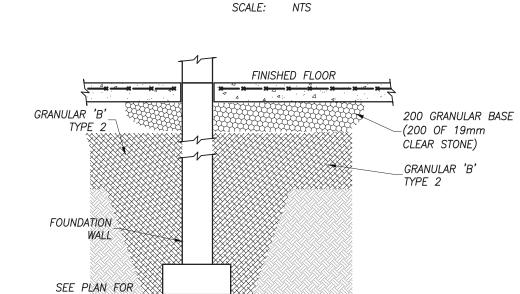
TOTAL



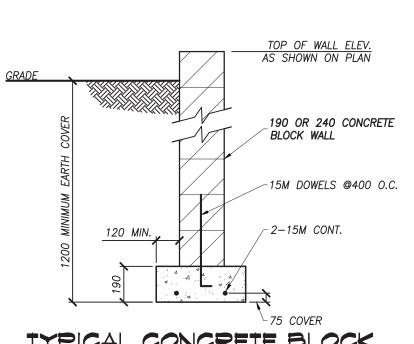


WHERE FTGS ARE ADJACENT TO

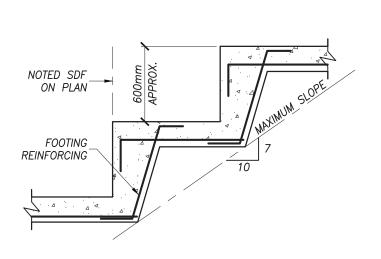
## ELEVATIONS OF ADJACENT FOOTINGS AND EXCAYATIONS



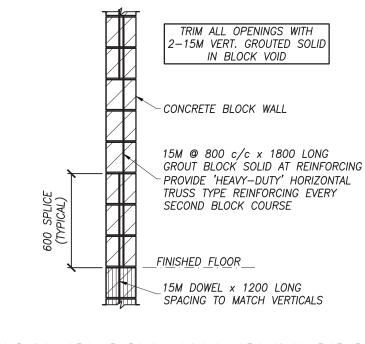
U/S FOOTING ELEV. (REQUIRED BOTH SIDES OF ALL INTERIOR AND EXTERIOR FOUNDATION WALLS) NOTE: NATIVE MATERIAL IS UNSUITABLE FOR BACKFILL TYPICAL EXCAVATION AND BACKFILL AT FOUNDATION WALLS SCALE: NTS



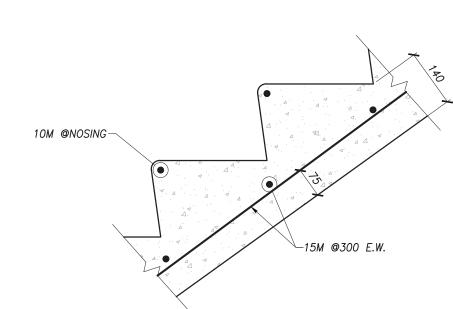
TYPICAL CONCRETE BLOCK STRIP FOOTING SCALE: NTS



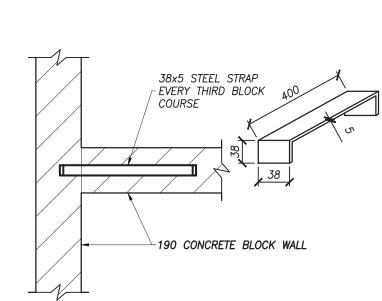
typical stepping of WALL FOOTING DETAIL



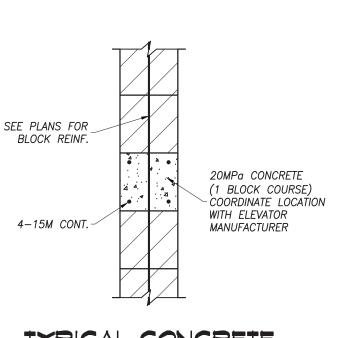
TYPICAL BLOCK WALL REINFORCING



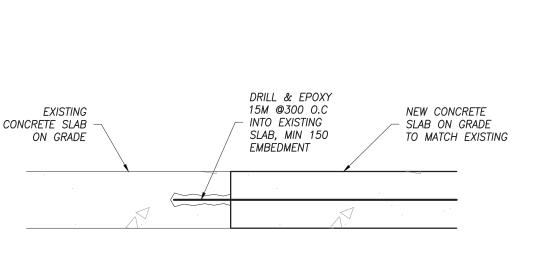
TYPICAL CAST-IN-PLACE STAIR REINFORCING SCALE: NTS



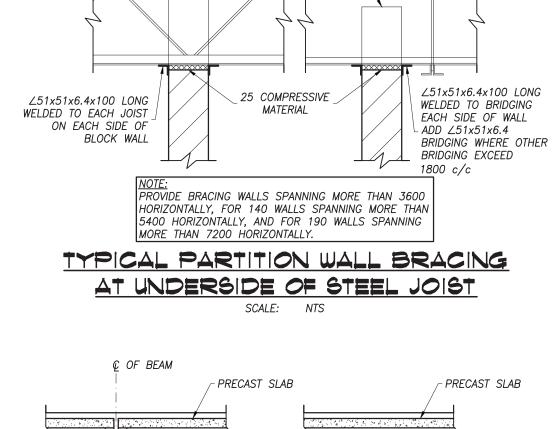
TYPICAL INTERSECTION OF CONCRETE BLOCK WALLS SCALE: NTS



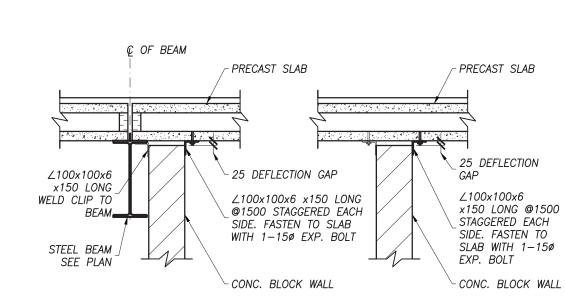
TYPICAL CONCRETE BOND BEAM SCALE: NTS



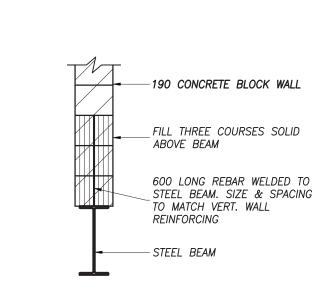
TYPICAL SLAB REPAIR DETAIL



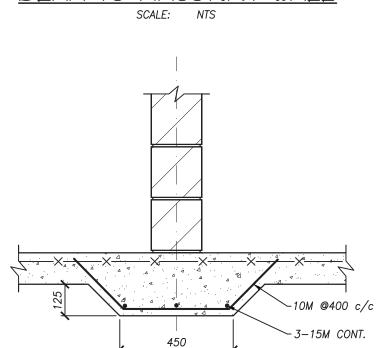
/ WALL BETWEEN JOISTS -



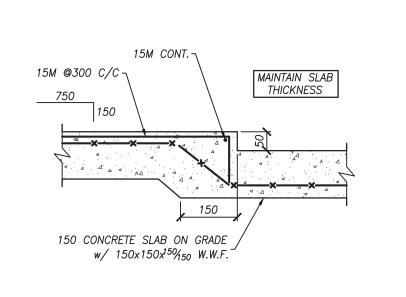
TYPICAL PARTITION WALL BRACING AT UNDERSIDE OF PRECAST SLAB SCALE: NTS



TYPICAL ANCHORAGE OF STEEL BEAM TO MASONRY WALL

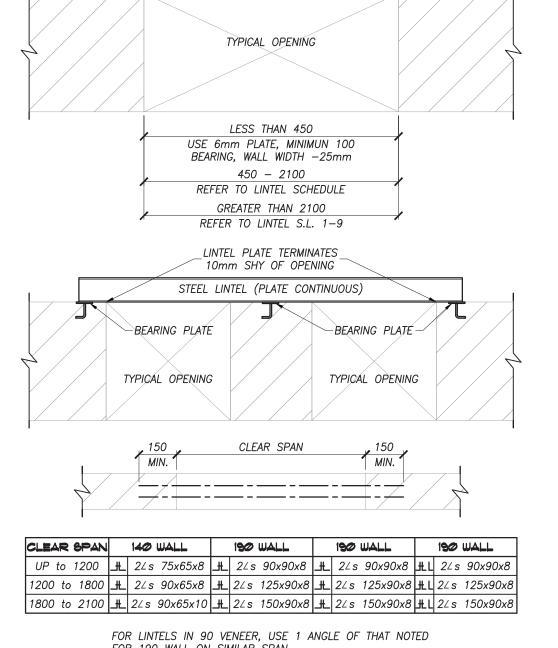


TYPICAL THICKENING OF SLAB ON GRADE UNDER PARTITION WALLS SCALE: NTS

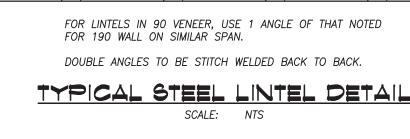


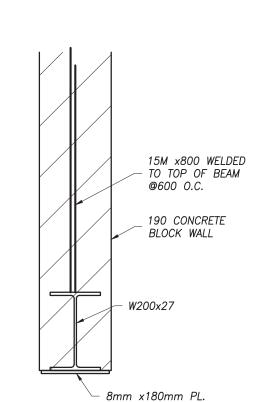
TYPICAL DETAIL AT FLOOR

DEPRESSION

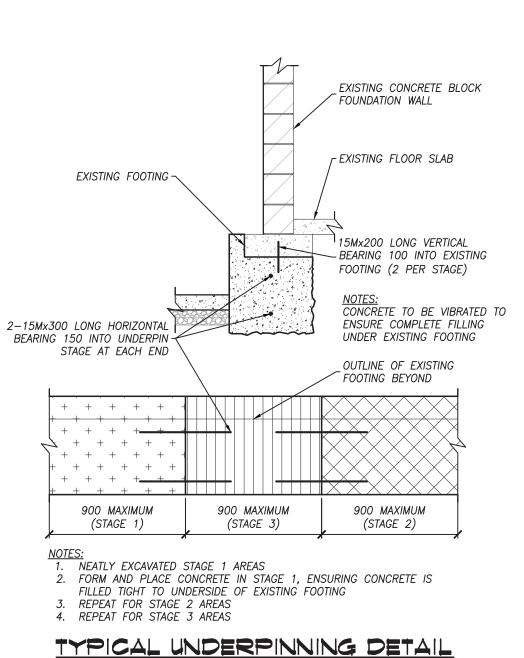


STEEL LINTEL

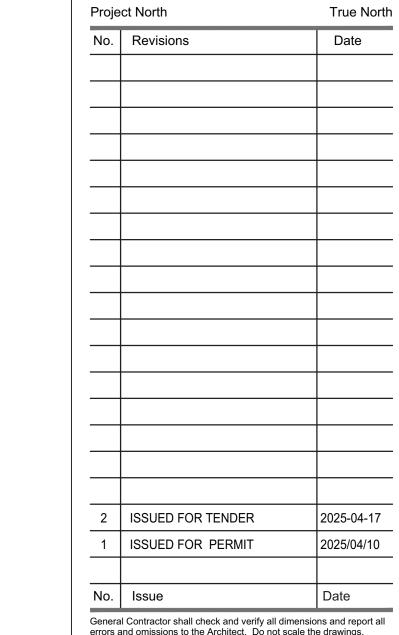




STEEL LINTEL S.L.1



SCALE: NTS



N ====

Halton District School Board

2050 Guelph Line

Burlington, Ontario

T. A. BLAKELOCK H.S.

RENOVATIONS

1160 Rebecca Street

Oakville, ON

Architect

Snyder Architects Inc.

100 Broadview Ave, Suite 301, Toronto, ON, M4M 3H3

tel. 416.966.5444 fax. 416.966.4443

www.snyderarchitects.ca

Consultants

Structural Consultants

300 York Boulevard

Hamilton, Ontario, L8R 3K6

Mechanical and Electrical Consultants

EXP

1266 S. Service Rd,

Stoney Creek, Ontario, L8E 5R9

Tel: 905-525-6069

Tel: 905-333-9119

Kalos Engineering Inc

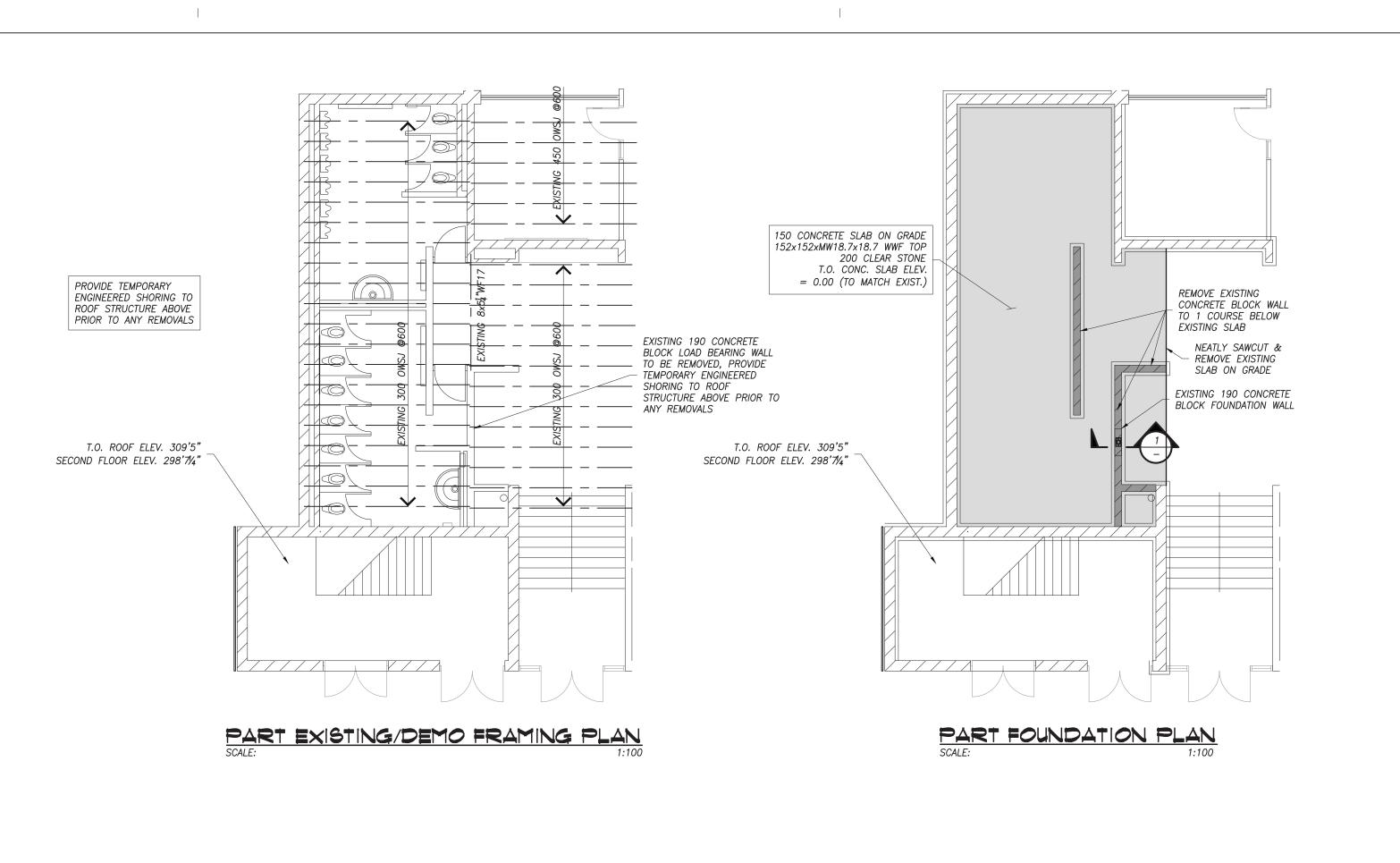


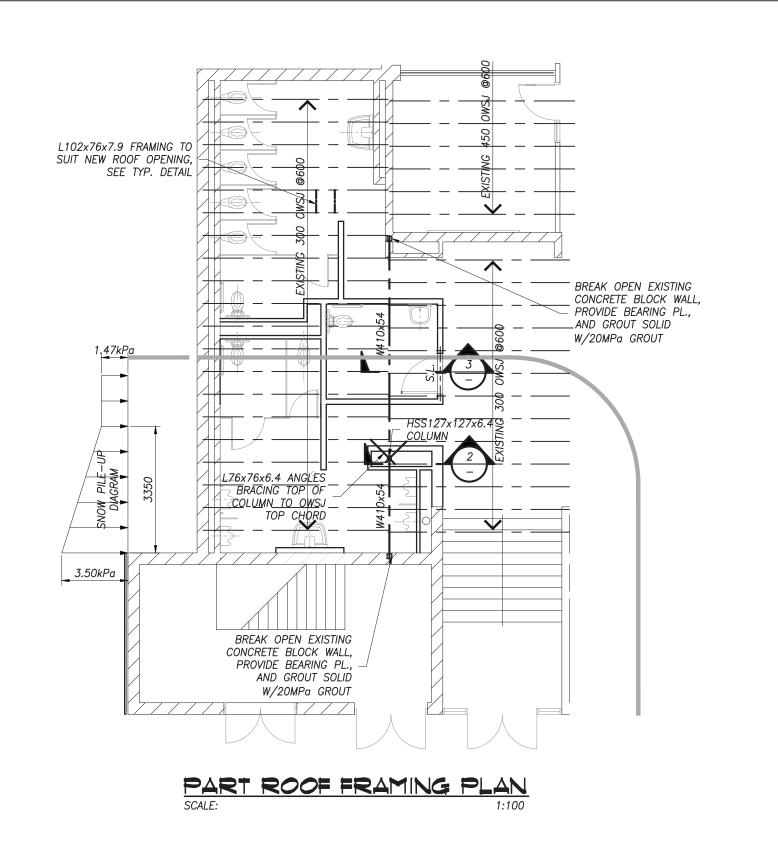
AS NOTED Date: MARCH 2025 Scale: 2215-C

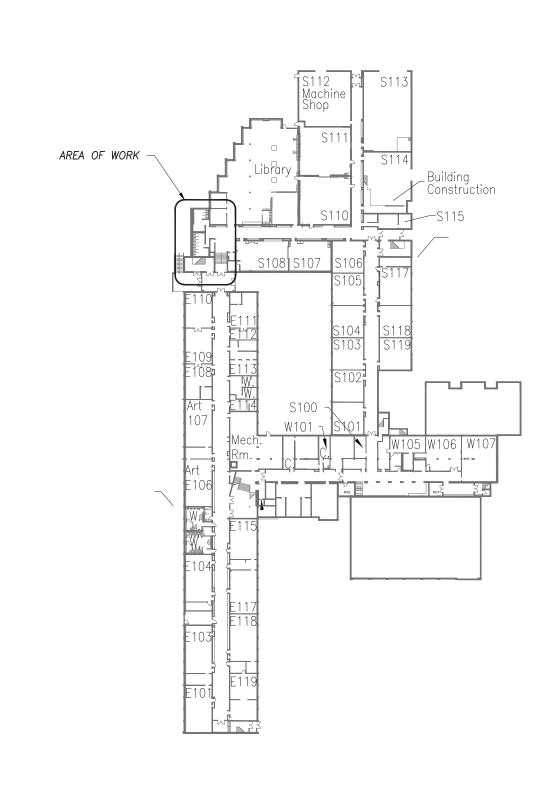
Drawings shall not be used for construction purposes until issued by the

Architect for construction.

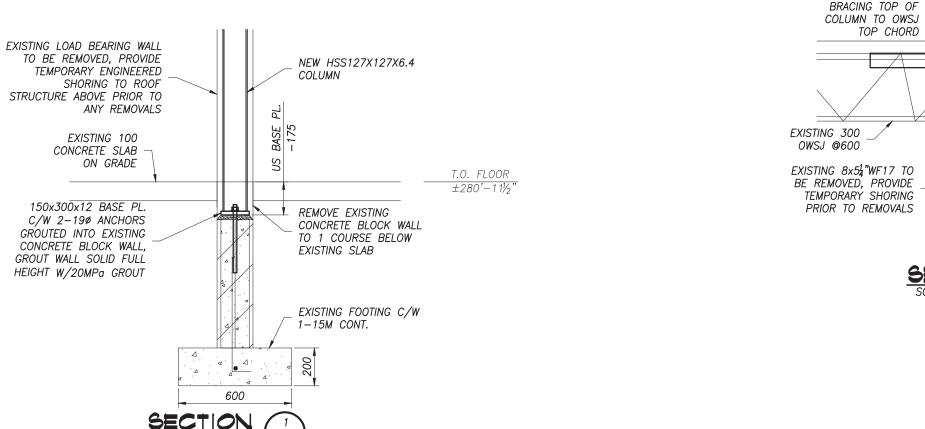
**TYPICAL** 

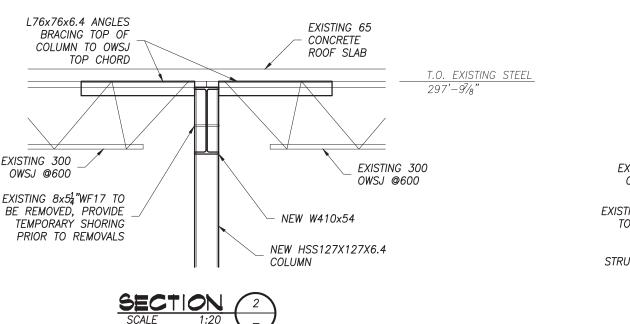


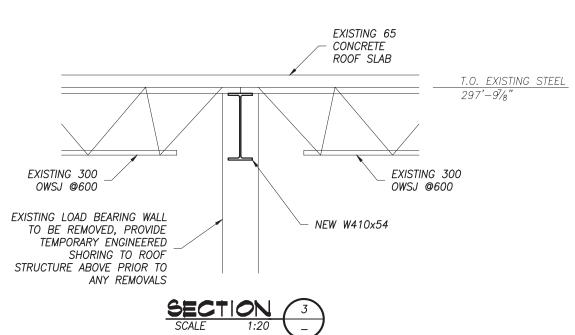


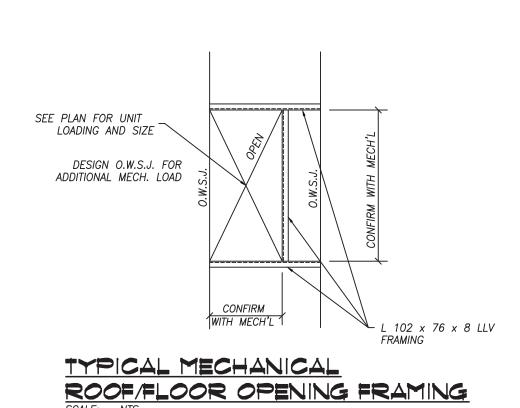


KEY PLAN - GROUND FLOOR FRAMING









## SHORING NOTES

- 1. ALL FRAMES AND SHORING JACKS TO BE PLUMB AND LEVEL.
- 2. SHORING JACKS TO BE DESIGNED BY THE SUPPLIER FOR THE LOADS AND HEIGHTS SHOWN, INCLUDING BRACING. 3. MAX. EXTENSION OF SCREWJACKS WILL BE 400mm UNLESS NOTED.
- 4. SCAFFOLDING SHALL BE ERECTED IN ACCORDANCE TO C.S.A. CODE S269.1.
- 5. SHORING TO REMAIN IN PLACE UNTIL BEAM AND ALL BRACING IS COMPLETELY INSTALLED.
- 6. CONTRACTOR TO PREPARE AND SUBMIT FULL SHORING DRAWINGS FOR APPROVAL FOR ALL TEMPORARY SUPPORTS. PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER, I
- ADDITIONAL NOTES: - PROVIDE TEMPORARY SHORING REMOVALS.

	Y A PROFESSIONAL ENGINEER, PRIOR TO ANY REMOVALS.	
<u>ADDI</u>	IONAL NOTES:	
-	PROVIDE TEMPORARY SHORING TO STRUCTURE ABOVE PRIOR TO ANY REMOVALS.  EXISTING FRAMING SHOWN IS ASSUMED BASED ON SITE REVIEW AND EXISTING DRAWINGS REFERENCED IN NOTES BELOW. CONTRACTOR TO EXPOSE EXISTING STRUCTURE AND REPORT ANY DISCREPANCIES TO THE ENGINEER.	

_	ect North	True
No.	Revisions	Date
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2	ISSUED FOR TENDER	2025-04
1	ISSUED FOR PERMIT	2025/04
No.	Issue	Date

**Halton District School Board** 2050 Guelph Line

Burlington, Ontario

T. A. BLAKELOCK H.S.

RENOVATIONS

1160 Rebecca Street

Oakville, ON

Snyder Architects Inc.

100 Broadview Ave, Suite 301, Toronto, ON, M4M 3H3 tel. 416.966.5444 fax. 416.966.4443

www.snyderarchitects.ca

Consultants

Structural Consultants

Kalos Engineering Inc 300 York Boulevard Hamilton, Ontario, L8R 3K6 Tel: 905-333-9119

Mechanical and Electrical Consultants

EXP 1266 S. Service Rd,

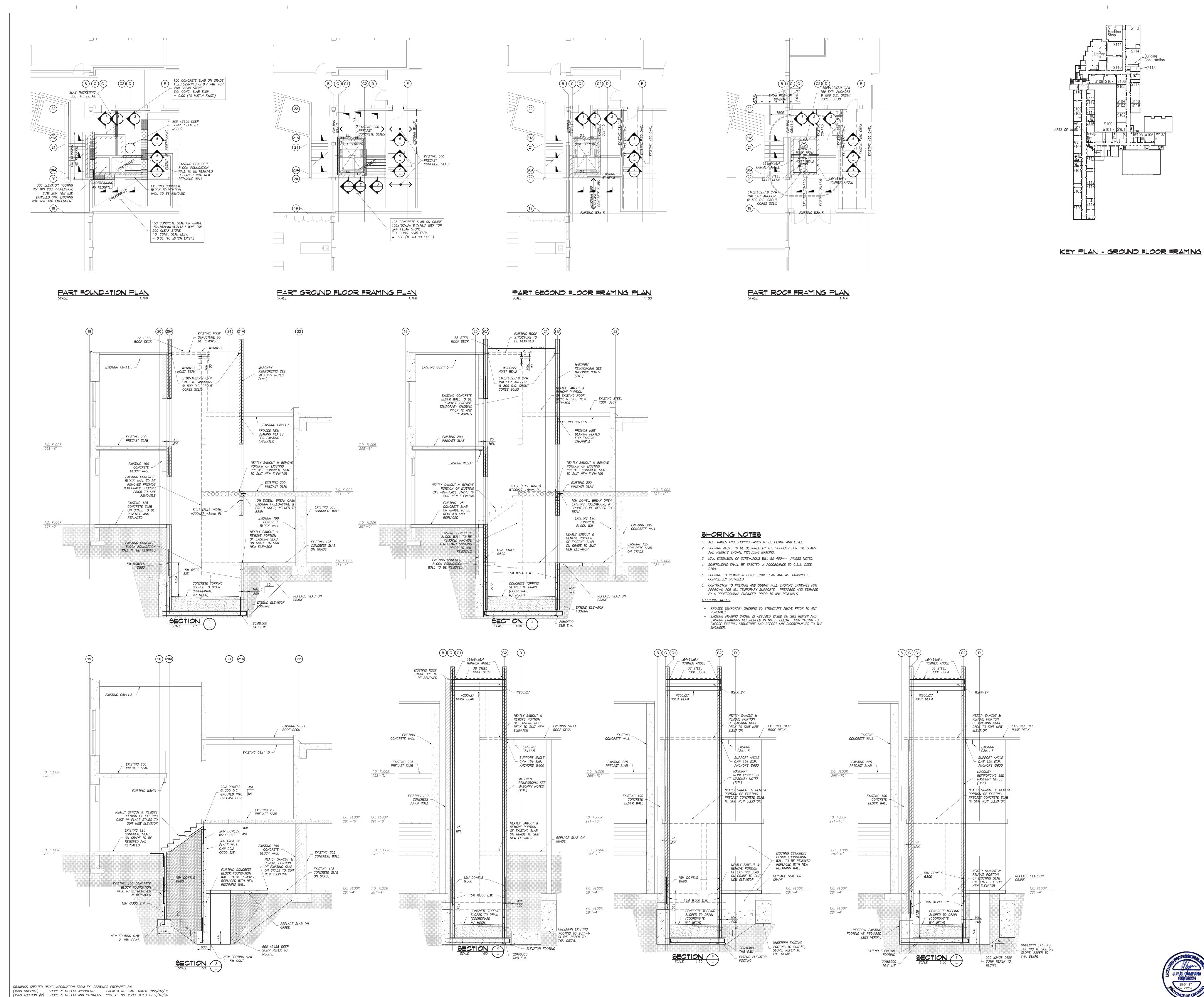
Stoney Creek, Ontario, L8E 5R9

Tel: 905-525-6069



RENOVATION DETAILS Scale: AS NOTED Date: MARCH 2025

Drawing Title:
WASHROOM



(1989 ADDITION #3) SVEDAS KOYANAGI ARCHITECTS & PHILIPS ENGINEERING. PROJ. NO. 89802, DATED OCT. 1989

**Halton District School Board** 2050 Guelph Line Burlington, Ontario

T. A. BLAKELOCK H.S. RENOVATIONS 1160 Rebecca Street

Oakville, ON

Snyder Architects Inc. 100 Broadview Ave, Suite 301, Toronto, ON, M4M 3H3 tel. 416.966.5444 fax. 416.966.4443 www.snyderarchitects.ca

> Consultants Structural Consultants Kalos Engineering Inc 300 York Boulevard Hamilton, Ontario, L8R 3K6

> > Tel: 905-525-6069

Tel: 905-333-9119 Mechanical and Electrical Consultants EXP 1266 S. Service Rd, Stoney Creek, Ontario, L8E 5R9

Drawing Title:
WEST ELEVATOR **DETAILS** 

ISSUED FOR TENDER

ISSUED FOR PERMIT

General Contractor shall check and verify all dimensions and report all

Drawings shall not be used for construction purposes until issued by the Architect for construction.

No. Issue

Project North

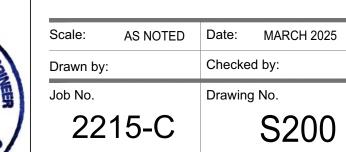
No. Revisions

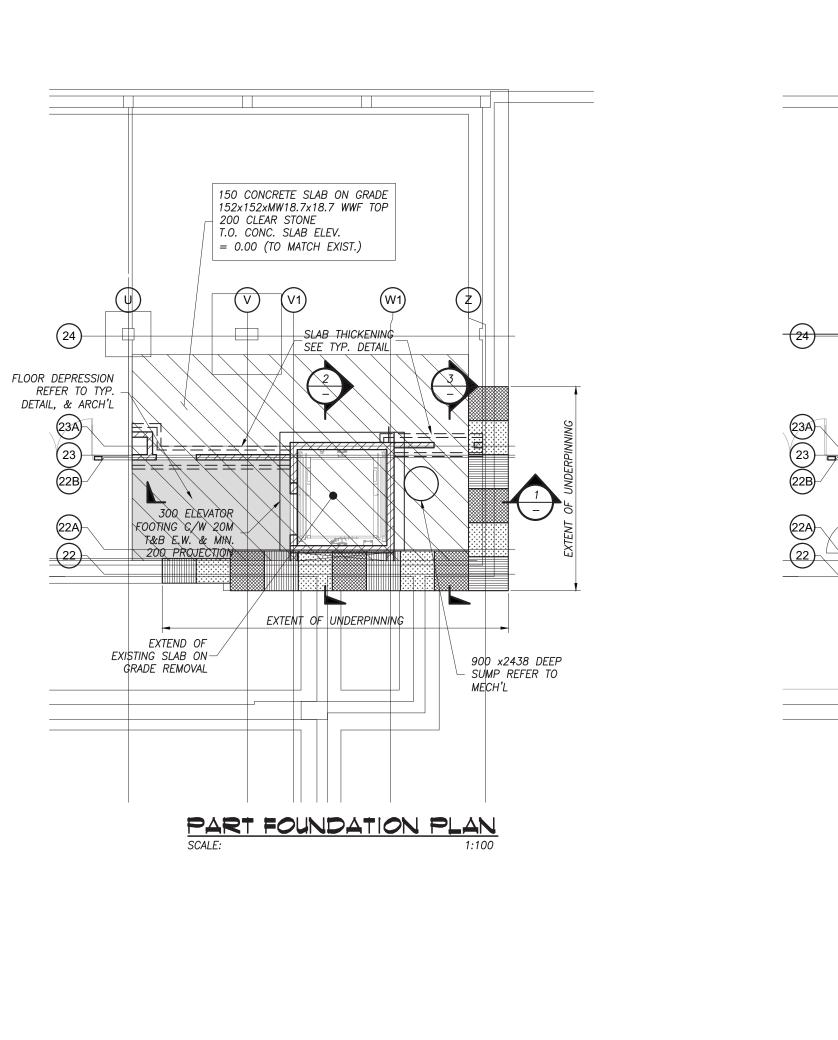
True North

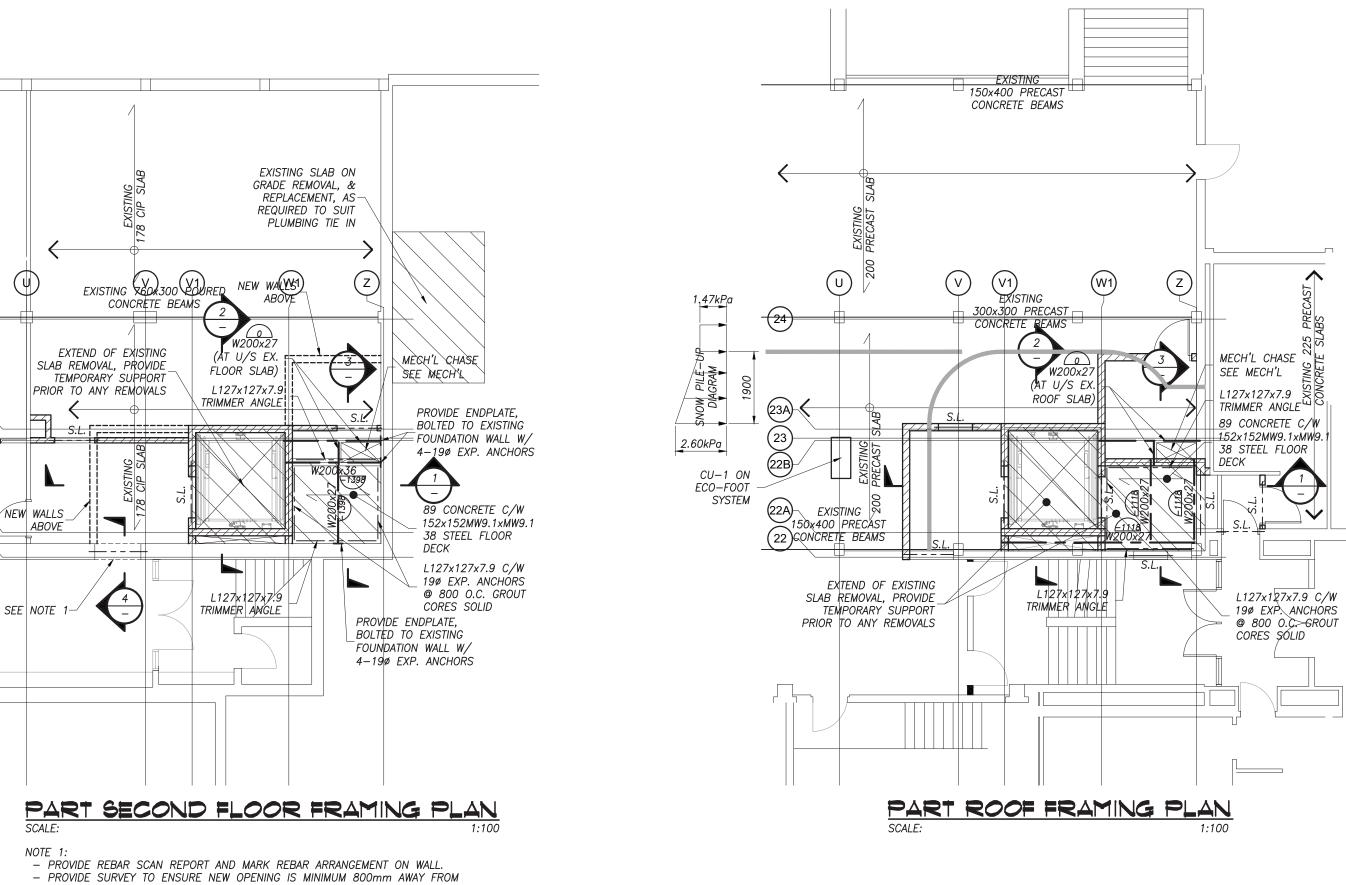
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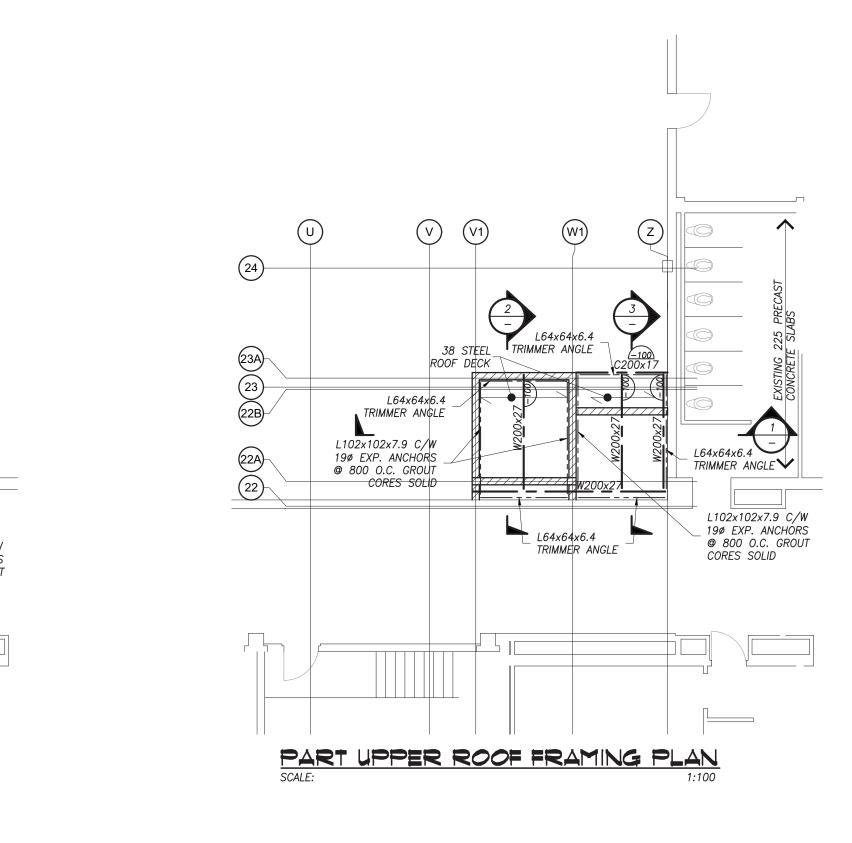
2025-04-17

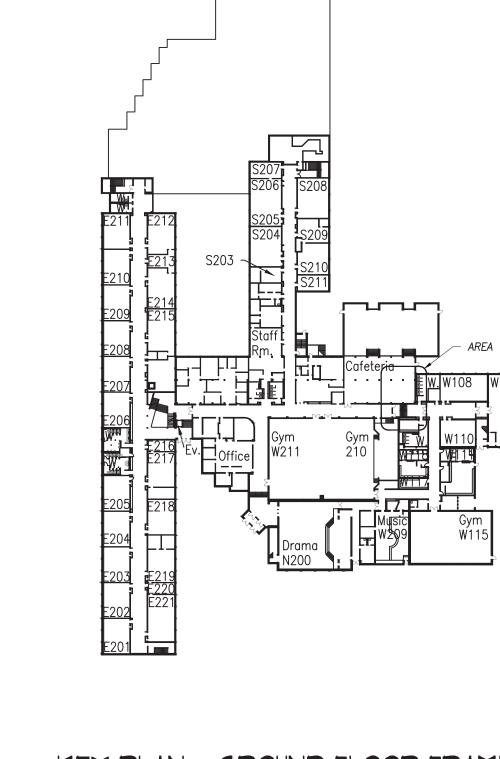
2025/04/10



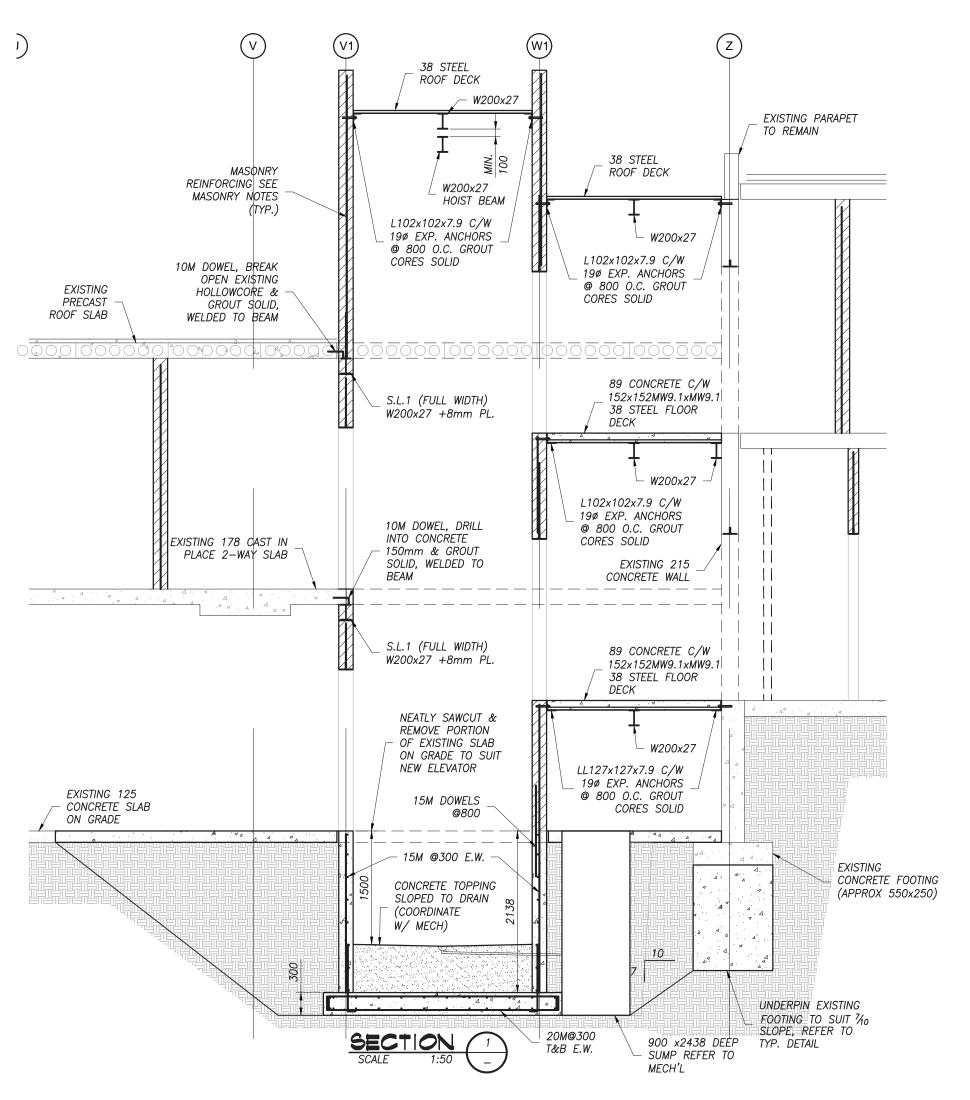








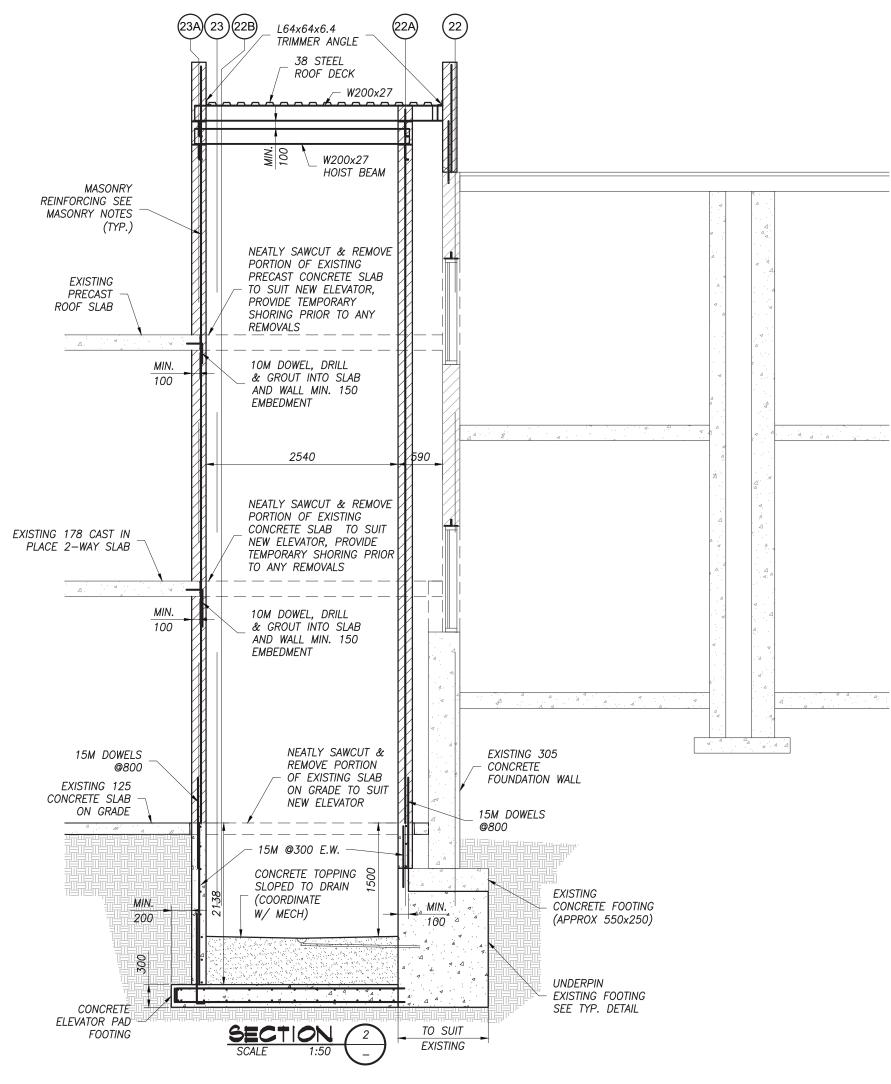
KEY PLAN - GROUND FLOOR FRAMING

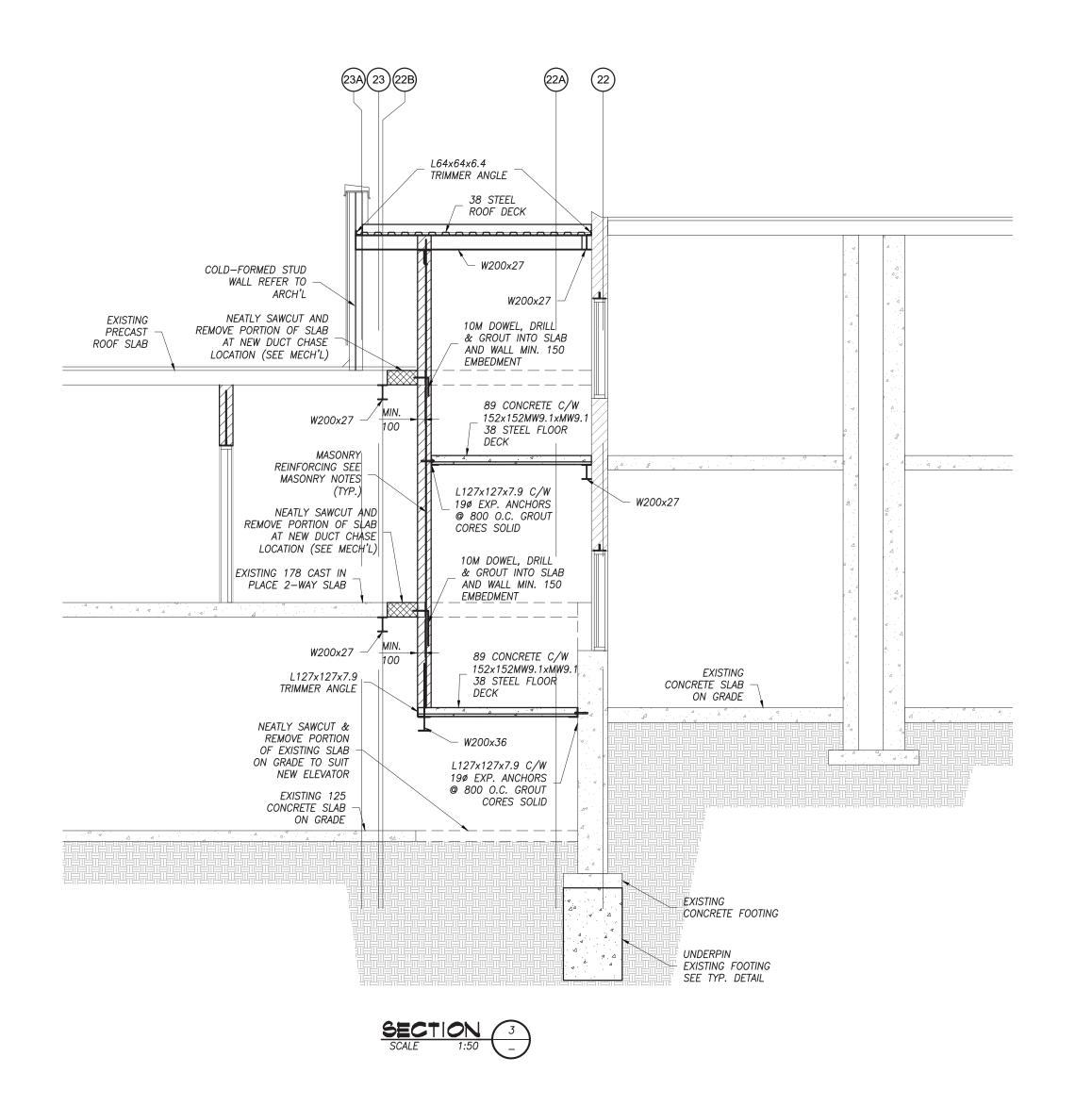


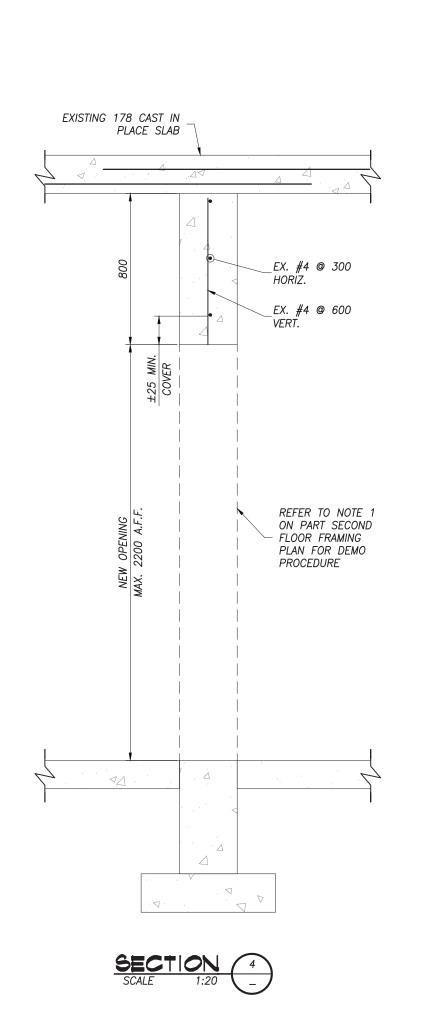
EXISTING COLUMN ABOVE.

AFFECTED SURFACES.

— SAW—CUT AND DISPOSE OF EXISTING CONCRETE WALL (UP TO NEW LINTEL) TO CREATE NEW OPENING MAX. HEIGHT 2200 A.F.F., MAKE THE EDGE SMOOTH TO FORM NEW CLEAN EDGE. SCAN REBAR PRIOR TO DEMOLITION ENSURE MIN. 25mm COVER OF EXISTING REBAR PRIOR TO SAW—CUTTING. COORDINATE W/ ARCH., MECH., ELEC., DWGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK. MAKE GOOD







## SHORING NOTES

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Stoney Creek, Ontario, L8E 5R9

Tel: 905-525-6069

Kalos Engineering Inc

Drawing Title:
EAST ELEVATOR
DETAILS
DLIAILS



Scale: AS NOTED Date: MARCH 2025

Drawn by: Checked by:

Job No. Drawing No.

2215-C S300