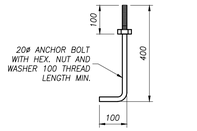
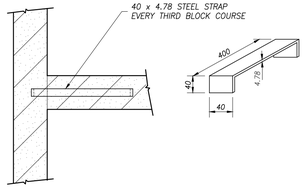
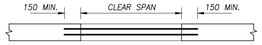


TYPICAL ANCHOR BOLT DETAIL
NOT TO SCALE

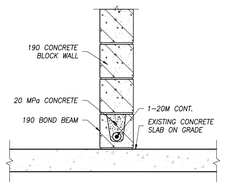


TYPICAL INTERSECTION OF CONCRETE BLOCK WALLS
NOT TO SCALE

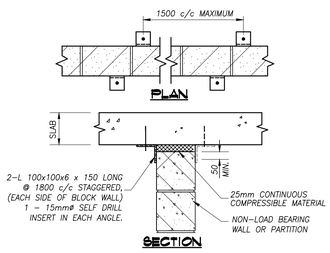


CLEAR SPAN	HEADER SIZE
UP TO 1200mm	2–90 x 16 ga (1.52mm)
1200 TO 1400mm	2–100 x 16 ga (1.52mm)
1400 TO 2000mm	2–150 x 16 ga (1.52mm)
2000 TO 2800mm	2–200 x 14 ga (1.91mm)

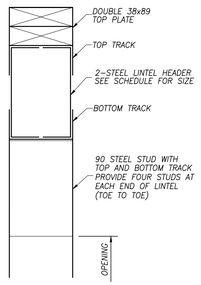
TYPICAL STEEL STUD LINTEL DETAIL
NOT TO SCALE



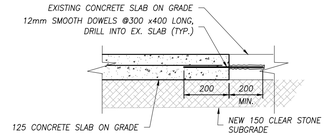
BOND BEAM ON SLAB ON GRADE
NOT TO SCALE



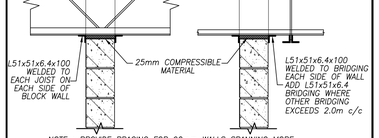
BRACING OF TOP OF WALL AT UNDERSIDE OF CONCRETE SLAB
NOT TO SCALE



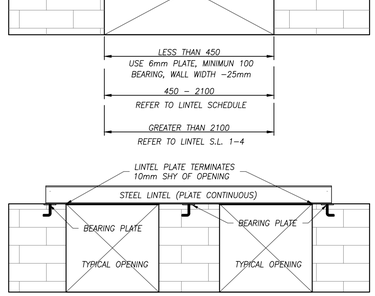
TYPICAL LINTEL SECTION
NOT TO SCALE

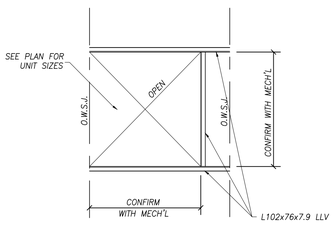


TYP. SLAB CONNECTION DETAIL
NOT TO SCALE



BRACING OF PARTITION WALLS AT UNDERSIDE OF STEEL JOIST
NOT TO SCALE





TYPICAL REINFORCING AT DUCT ROOF OPENING
NOT TO SCALE

(REFER TO MECHANICAL FOR LOCATIONS)

CLEAR SPAN	140 WALL	190 WALL
UP to 1200	2Ls 75x65x8	2Ls 90x90x8
1200 to 1800	2Ls 90x65x8	2Ls 125x90x8
1800 to 2100	2Ls 90x65x10	2Ls 150x90x8

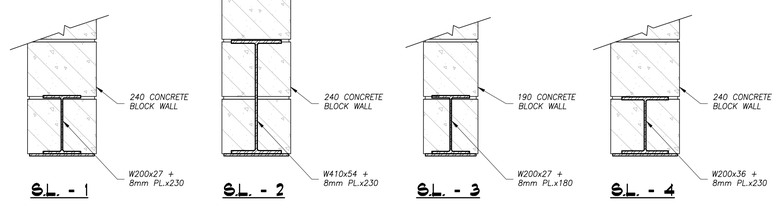
CLEAR SPAN	240 WALL	290 WALL
UP to 1200	2Ls 100x100x8	3Ls 90x90x8
1200 to 1800	2Ls 150x100x8	3Ls 125x90x8
1800 to 2100	2Ls 150x100x8	3Ls 150x90x8

FOR LINTELS IN 90 VENEER, USE 1 ANGLE OF THAT NOTED FOR 190 WALL ON SIMILAR SPAN.

DOUBLE ANGLES TO BE STITCH WELDED BACK TO BACK.

TYPICAL STEEL LINTEL DETAIL
NOT TO SCALE

(REFER TO MECHANICAL FOR OPENING SIZE & LOCATIONS FOR LINTELS ABOVE MECH. PENETRATIONS)



GENERAL NOTES

1. CHECK ALL DIMENSIONS ON THESE DRAWINGS WITH ALL OTHER DRAWINGS, INCLUDING BUT NOT LIMITED TO DRAWINGS PREPARED ARCHITECTURAL, MECHANICAL OR ELECTRICAL CONSULTANTS. REPORT ANY INCONSISTENCIES TO THE ENGINEER PRIOR TO COMMENCING WITH THE WORK. DO NOT SCALE THE DRAWINGS.
2. THE DESIGN LIVE LOADS ARE INDICATED ON THE DRAWINGS. RENOVATION LOADS SHALL NOT EXCEED THE DESIGN LOADS.
3. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING AND ANY OTHER TEMPORARY OR PERMANENT MEASURES AS REQUIRED DURING RENOVATION. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY SUPPORT OF EXISTING OR ADJACENT STRUCTURES AS REQUIRED. ALL BRACING AND SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
4. REFER TO OTHER CONSULTANTS DRAWINGS FOR DETAILS NOT INDICATED ON THE STRUCTURAL DRAWINGS.
5. THE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS.
6. CLARIFY ANY QUERIES WITH THE ENGINEER REGARDING THE INTERPRETATION OF THE DRAWINGS, PRIOR TO THE COMMENCEMENT OF ANY WORK.

CONCRETE NOTES

1. ALL STRUCTURAL CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.3. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.1.
2. MINIMUM CONCRETE STRENGTH AT 28 DAYS SHALL BE:
   - FOOTINGS 25 MPa TYPE N
   - SLAB ON GRADE 25 MPa TYPE N

   SLUMP SHALL BE 75mm± 25mm. AGGREGATE SHALL BE 20mm MAXIMUM. AIR ENTRAINED TO BE 6% ± 1% WHEN EXPOSED TO EXTERIOR. CONTRACTOR TO SUBMIT CONCRETE MIX DESIGN FOR REVIEW.
3. THE DEFORMED REINFORCING STEEL SHALL CONFORM TO CSA STANDARD G30.18M GRADE 300R FOR STIRRUPS AND TIES AND GRADE 400R FOR ALL OTHER REINFORCING. UNLESS OTHERWISE NOTED THE REINFORCING LAP LENGTH SHALL BE 'CLASS B' IN SPLICES. ALL REINFORCING HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH A23.1.
4. WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH CSA G30.5. ALL MESH SHALL BE CHAIRED PRIOR TO THE CONCRETE POUR. LIFTING OF THE MESH DURING THE CONCRETE POUR WILL NOT PERMITTED. ALL SPLICES SHALL BE A MINIMUM OF TWO CROSS WIRE SPACINGS PLUS 50mm.
5. THE REINFORCING COVER FOR CONCRETE SHALL BE:
   - 75mm FOR CONCRETE AGAINST EARTH
   - 40mm FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 15M OR SMALLER
   - 50mm FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 20M OR LARGER
   - 25mm FOR INTERIOR CONCRETE. ALL CHAIRS, BOLSTERS, SPACERS AND BAR SUPPORTS SHALL BE IN ACCORDANCE WITH A23.1.
6. FOOTINGS SHALL BEAR ON NATIVE UNDISTURBED SOIL WITH A MINIMUM BEARING RESISTANCE OF:
   - 100 kPa (SLS)
   - 150 kPa (ULS)

   THE CONTRACTOR SHALL VERIFY THE CAPACITY PRIOR TO PLACEMENT OF CONCRETE.
7. THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATION OR STEP DOWN FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10. STEP HEIGHT SHALL NOT EXCEED 600mm.
8. KEEP EXCAVATIONS DRY BEFORE CONCRETE IS PLACED. REMOVE ALL LOOSE MATERIAL, SOFT SOIL OR WATER PRIOR TO PLACING CONCRETE. PROVIDE A 75mm MUD SLAB FOR ALL FOOTINGS BELOW THE WATER TABLE.
9. THE FOOTING DESIGN IS BASED ON INFORMATION AVAILABLE AT THE TIME OF DESIGN. THE FOOTING DESIGN MAY BE ALTERED DURING CONSTRUCTION, IF THE SITE CONDITIONS WARRANT, BUT ONLY WITH THE EXPRESS PERMISSION OF THE ENGINEER.
10. ALL ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE HILTI HIT–HY200 (OR APPROVED EQUAL) PROCEDURES.

LIGHT GAUGE STEEL FRAMING NOTES

GENERAL

1. THESE NOTES APPLY TO THE STEEL STUD FRAMING COMPONENT OF THE EXTERIOR WALL SYSTEM ONLY.
2. THE DESIGN WIND LOADING IS 1.2kN/m² (25 PSF) DETERMINED BY O.B.C. REQUIREMENTS AND CAN–S136. DEFLECTION IS LIMITED TO L/360.
3. THE DESIGN OF FRAMING SYSTEM IS BASED ON PUBLISHED STUD SECTION PROPERTIES BY BAILEY METAL PRODUCTS LIMITED.

MATERIAL

1. THE MINIMUM BASE METAL THICKNESS FOR ALL METAL WALL COMPONENTS, EXCLUDING COATINGS ARE NOTED ON THE DRAWINGS.
2. STEEL MEETS THE REQUIREMENTS OF A.S.T.M. A653/A653M SS GRADE 33 (230) FOR 1.22mm MATERIAL AND THINNER, AND SS GRADE 50 (340) CLASS 1 FOR 1.52mm MATERIAL AND THICKER.
3. GALVANIZING TO BE HOT–DIP PROCESS, G90 (Z275).

EXECUTION

1. METHOD OF CONSTRUCTION SHALL BE BY STICK BUILDING ON SITE.
2. CONNECTIONS SHALL BE ACCOMPLISHED BY SELF DRILLING SCREWS AND OTHER FASTENERS AS SHOWN ON THESE DRAWINGS. PENETRATION BEYOND JOINED MATERIALS SHALL BE NOT LESS THAN THREE EXPOSED THREADS. ALL CONNECTORS USED IN ASSEMBLIES SHALL BE OF CORROSION RESISTANT MATERIAL COMPATIBLE WITH GALVANIZED COATINGS WITH A MINIMUM COATING THICKNESS OF 0.039mm ZINC OF CADMIUM PLATES. NO BLACK CONNECTORS WILL BE ACCEPTED. SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER.
3. SCREWS COVERED BY SHEATHING MATERIALS SHALL HAVE LOW PROFILE HEADS.
4. WIRE TYING IS NOT PERMITTED IN STRUCTURAL APPLICATIONS.
5. CUTTING OF STEEL FRAMING MEMBERS SHALL BE BY SAW OR SHEAR. NO TORCH OR MANUAL CUTTING IS PERMITTED.
6. SPLICING OF STUDS OR TRACK IS NOT PERMITTED EXCEPT AS NOTED ON DRAWINGS.
7. BRIDGING SHALL BE OF SIZE, SPACING AND TYPE SHOWN ON THE DRAWINGS AND SHALL BE INSTALLED SO AS TO PROVIDE RESISTANCE TO MINOR AXIS BENDING AND ROTATION OF STUDS. PROVIDE BRIDGING AT 1200mm c/c MAXIMUM.
8. TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.
9. STUDS SHALL SEAT INTO TOP AND BOTTOM TRACKS WITH THE GAP BETWEEN THE END OF THE STUD AND WEB OF THE TRACK NOT TO EXCEED 3mm.
10. VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/1000 OF THE SPAN.
11. HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/1000 OF THEIR RESPECTIVE LENGTHS.
12. SPACING OF STUDS SHALL BE WITHIN 3mm FROM DESIGN SPACING PROVIDED THAT CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF THE FINISHING MATERIALS.

MASONRY NOTES

1. ALL STRUCTURAL ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD S304.1. ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH CSA STANDARD A371. ALL MASONRY CONNECTORS, REINFORCING AND TYING SHALL BE IN ACCORDANCE WITH CSA A370. ALL MORTAR AND GROUT SHALL BE IN ACCORDANCE WITH A179.
2. ALL CONCRETE BLOCKS SHALL BE NORMAL WEIGHT TYPE H/15/A/M UNLESS OTHERWISE NOTED. MORTAR SHALL BE TYPE S FOR LOADBEARING AND TYPE N FOR NON–LOADBEARING.
3. VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 6000mm. REFER TO ARCHITECTURAL DRAWING FOR DETAILS AND LOCATIONS.
4. TRIM ALL OPENINGS WITH 2–15M BARS.
5. GROUT SHALL CONSIST OF ON ONE PART PORTLAND CEMENT, THREE PARTS SAND (MAXIMUM AGGREGATE SIZE SHALL BE 10mm) WITH WATER TO PROVIDE A MINIMUM 10MPa COMPRESSIVE STRENGTH AT 28 DAYS. SLUMP SHALL BE 200mm TO 250mm.
6. ALL CELLS CONTAINING REINFORCING SHALL BE GROUTED SOLID. TWO BLOCK COURSES BELOW BEARING PLATES SHALL BE GROUTED SOLID.
7. THE MASONRY SHALL BE CONSTRUCTED EVENLY WITH MAXIMUM LIFTS OF 1200 PER DAY. DO NOT TOOTH AND BOND OR STACK BOND MASONRY. RAKE BACK ENDS OF UNFINISHED WALLS.
8. ALL MORTAR JOINTS SHALL BE TOOLED (CONCAVE). A MINIMUM BED JOINT OF 6mm IS REQUIRED FOR THE STARTING COURSE TO A MAXIMUM OF 20mm. THE BED JOINTS SHALL BE 10mm.
9. PROVIDE VERTICAL AND HORIZONTAL REINFORCING AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
   - 140 CONCRETE BLOCK – 10M VERTICAL AT 600 O.C. & HEAVY DUTY TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE.
   - 190 CONCRETE BLOCK – 15M VERTICAL AT 800 O.C. & HEAVY DUTY TRUSS TYPE HORIZONTAL REINFORCING EVERY SECOND COURSE.
   - 240 CONCRETE BLOCK – 20M VERTICAL AT 600 O.C. & HEAVY DUTY TRUSS TYPE HORIZONTAL REINFORCING EVERY COURSE.
10. THE HORIZONTAL REINFORCING AT EXTERIOR WALLS SHALL BE GALVANIZED. DO NOT EXTEND HORIZONTAL REINFORCING THROUGH CONTROL JOINTS UNLESS OTHERWISE NOTED.
11. PROVIDE A STEEL LINTEL OVER ALL OPENINGS OR RECESSES INCLUDING OPENINGS FOR MECHANICAL AND ELECTRICAL COMPONENTS. ALL EXTERIOR LINTELS TO BE HOT DIP GALVANIZED.
12. BUILD THE MASONRY SOLID AROUND ALL BEAM, LINTEL AND JOIST POCKETS. INSTALL BEARING PLATES AT THE SPECIFIED ELEVATION AND GROUT THE PLATE INTO THE WALL A MINIMUM OF 400mm.
13. PROVIDE TEMPORARY BRACING AS REQUIRED TO SUPPORT THE MASONRY WALLS IN CONSTRUCTION. PROTECT THE MASONRY WALLS FROM THE ELEMENTS AT ALL TIMES EXCEPT DURING CONSTRUCTION PROGRESS.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL ELEMENTS, INCLUDING DESIGN OF ELEMENTS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH CAN/CSA S16.
2. ALL STRUCTURAL STEEL SHALL CONFORM TO CSA G40.21 (300W) EXCEPT W SECTIONS AND PLATES G40.21 (350W), HSS MEMBERS G40.21 (350W) CLASS C OR ASTM A500 GRADE C, ANCHOR BOLTS ASTM A307, COLD FORMED SECTIONS ASTM A570M GRADE 350W. UNLESS OTHERWISE NOTED, ALL SECTIONS SHALL BE PRIME PAINTED WITH THE SURFACE PREPARATION AND PAINTING PROCEDURES IN ACCORDANCE WITH CAN/CGSB 85.10.
3. ALL WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH CAN/CSA W59. THE STEEL FABRICATOR SHALL BE FULLY QUALIFIED UNDER THE REQUIREMENTS BY THE CANADIAN WELDING BUREAU IN CONFORMANCE WITH CAN/CSA W47.1.
4. DESIGN ALL MOMENT AND SHEAR CONNECTIONS FOR THE FULL CAPACITY OF THE SMALLER MEMBER IN THE CONNECTION UNLESS OTHERWISE NOTED.
5. PROVIDE MINIMUM BEARING LENGTH OF STEEL MEMBERS AS FOLLOWS:
   - ON MASONRY – 150mm
   - ON STEEL – 90mm
6. THE BASE PLATE AND BEARING PLATE GROUT SHALL BE OF THE CEMENTITIOUS NON–SHRINK TYPE.
7. DECK SHALL BE EITHER 38mm OR 76mm DEEP IN ACCORDANCE WITH CSA S136 AND SHALL BE FABRICATED FROM ASTM A653 SS GRADE 230 GALVANIZED STEEL WITH A ZF75 GALVANNEAL OR Z275 GALVANIZED ZINC COATING. THE MINIMUM NOMINAL STEEL CORE THICKNESS SHALL BE 0.76mm. STEEL DECK SHALL BE FASTENED TO THE SUPPORT STRUCTURE WITH 20mm SPOT WELDS AT NOT MORE THAN 300mm c/c (150mm AT PERIMETER). CLINCH SIDELAPS AT 600mm c/c. ALL WELDS TO BE TOUCHED UP WITH PRIMER. MECHANICAL FASTENERS MAY ONLY BE USED WITH THE PERMISSION OF THE ENGINEER.
8. FULLY WELD THE BASE PLATE TO THE COLUMN TO DEVELOP THE ANCHOR BOLTS. PROVIDE CAP PLATES ON ALL COLUMNS. PROVIDE 6mm CAP PLATES ON ALL COLUMNS.
9. PROVIDE MINIMUM 175x10x175 BEARING PLATES FOR ALL STRUCTURAL STEEL c/w 2–15Ø ANCHORS UNLESS OTHERWISE NOTED.
10. ALL BOLTS SHALL BE TIGHTENED WITH A SUITABLE TORQUE WRENCH IN ACCORDANCE WITH CSA S16.
11. ALL STEEL EXPOSED TO THE EXTERIOR TO BE HOT DIP GALVANIZED.
12. ERECT STRUCTURAL STEEL IN ACCORDANCE WITH CSA S16 AND IN CONFORMANCE WITH THE APPROVED SHOP DRAWINGS.

SUBMITTALS

1. SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: TEMPORARY SHORING.
2. THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN 1:50 (¼"=1'–0").
3. THE STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED, IN WHOLE OR IN PART, FOR USE AS SHOP DRAWINGS.
4. EACH DRAWING SUBMITTED FOR TEMPORARY SHORING SHALL BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
5. CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.

LOADING SUMMARY

DESIGN STANDARDS

- ONTARIO BUILDING CODE, 2012, PART 4: STRUCTURAL DESIGN
- CAN/CSA–A23.3–14, DESIGN OF CONCRETE STRUCTURES
- CAN/CSA–A23.4–16, DESIGN OF PRECAST CONCRETE STRUCTURES
- CAN/CSA–S304.1–14, MASONRY DESIGN FOR BUILDINGS
- CAN/CSA–S16–14, LIMIT STATES DESIGN OF STEEL STRUCTURES
- CAN/CSA–S136–16, DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS

SNOW, ICE AND RAIN LOADS

APPLIED PER OBC, PART 4, SECTION 4.1.6

- IMPORTANCE FACTOR, Is 0.9 (SLS) 1.15 (ULS)
- GROUND SNOW LOAD, Ss 1.1 kPa (22.97 PSF)
- ASSOCIATED RAIN LOAD, Sr 0.4 kPa (8.35 PSF)
- WIND EXPOSURE FACTOR, Cw, 1.0
- ROOF SNOW LOAD, S, 1.47 kPa (30.70 PSF)
- DRIFT LOADS PER CLAUSE 4.1.6.2.8
- SLOPE FACTORS PER CLAUSE 4.1.6.2.(5) TO (7)
- ROOF STRUCTURE TO BE DESIGNED IN ACCORDANCE WITH CLAUSE 7.4.10.4(2)

WIND LOADS

APPLIED PER OBC, PART 4, SECTION 4.1.7

- IMPORTANCE FACTOR, Iw 0.7 (SLS) 1.15 (ULS)
- REFERENCE VELOCITY PRESSURE FOR STRUCTURAL MEMBERS
  0.47 kPa 1/50 YEAR PROBABILITY (9.82 PSF)
- REFERENCE VELOCITY PRESSURE FOR CLADDING & NON–STRUCTURAL MEMBERS
  0.36 kPa 1/10 YEAR PROBABILITY (7.5 PSF)
- GUST FACTORS Cg:
  2.0 FOR WHOLE & MAIN STRUCTURAL MEMBERS
  2.5 FOR SMALL ELEMENTS INCLUDING CLADDING
  2.0 FOR INTERNAL PRESSURES.
- BUILDING INTERNAL PRESSURE CATEGORY 2 PER NBC 2010 STRUCTURAL COMMENTARY (PART 8), COMMENTARY B.

SEISMIC LOADS

APPLIED PER OBC, PART 4, SECTION 4.1.8

- IMPORTANCE FACTOR, Ie 1.3 (ULS)

Sa(0.2)	0.260
Sa(0.5)	0.129
Sa(1.0)	0.062
Sa(2.0)	0.029
PGA	0.167
SOIL CLASS	C (ASSUMED)
Fa	1.0

FLOOR LOADS

APPLIED PER OBC, PART 4, TABLE 4.1.5.3

- STAIRS 4.8 kPa (100 PSF)
- CORRIDORS 4.8 kPa (100 PSF)
- CLASSROOMS 2.4 kPa (50 PSF)

SEISMIC SWAY BRACING

ARTICLE 4.1.8.18(2) OF THE ONTARIO BUILDING CODE NOTES THAT IF THE PRODUCT OF Ie • Fa • Sa(0.2) IS LESS THAN 0.35, THE REQUIREMENTS NOTED ABOVE NEED NOT APPLY. THESE VALUES ARE EXPLORED BELOW. THIS EXEMPTION IS NOT APPLICABLE TO POST-DISASTER BUILDINGS.

BASED ON THE ABOVE NOTED VALUES, THE PRODUCT OF Ie • Fa * Sa(0.2) = 1.3 * 1.0 * 0.260 = 0.338 GIVEN THIS IS LESSER THAN THE THRESHOLD OF 0.35, THE APPLICATION OF THE LATERAL FORCE (Vp) TO ALL ELEMENTS AND COMPONENTS AND SWAY BRACING IS NOT REQUIRED.

DRAWINGS CREATED USING INFORMATION FROM EX. DRAWINGS PREPARED BY:
(1955 ORIGINAL) SHORE & MOFFAT ARCHITECTS. PROJECT NO. 230 DATED 1956/02/06
(1969 ADDITION #2) SHORE & MOFFAT AND PARTNERS. PROJECT NO. 230D DATED 1969/10/20

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Burlington, Ontario

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Key Plan N.T.S.

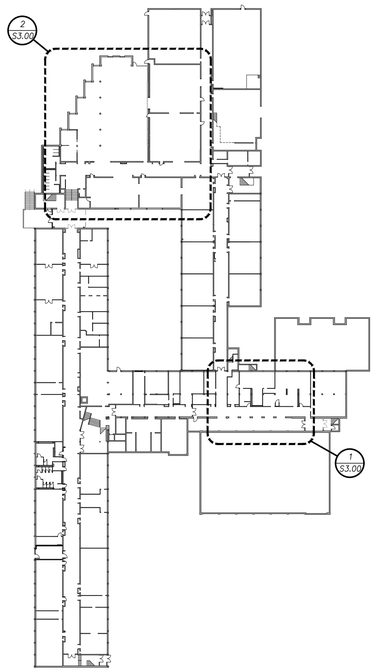
Project North True North

No.	Revisions	Date
1	ISSUED FOR BIDS / PERMIT	2023/03/13
No.	Issue	Date

LICENSED PROFESSIONAL ENGINEER
E. G. HADDAD
100210359
03/13/2023
PROVINCE OF ONTARIO

Drawing Title:
COVER PAGE, GENERAL NOTES & DETAILS

Scale:	AS NOTED	Date:	MARCH 2023
Drawn by:	JRD	Checked by:	EH
Job No.	2215A	Drawing No.	S0.01



LEVEL 0 - KEY PLAN
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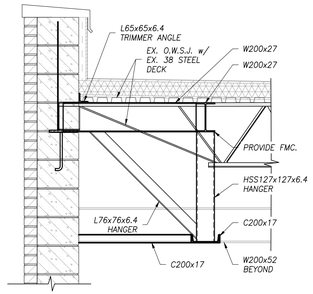
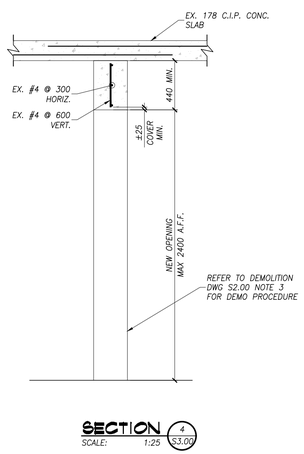


SECTION 3 S3.00
SCALE: 1:20



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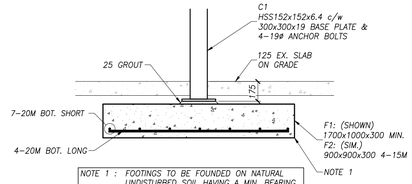

NOTE 1 : FOOTINGS TO BE FOUNDED ON NATURAL UNDISTURBED SOIL HAVING A MIN. BEARING CAPACITY AS IN THE GENERAL NOTES TO BE CONFIRMED BY A SOILS ENGINEER. REFER TO PLAN FOR APPROX. U/S OF FOOTING ELEVATION. INCREASE FOOTING THICKNESS AS REQUIRED TO MATCH U/S OF EX. FTG. ELEVATION.

SECTION 5 S3.00
SCALE: 1:25

INTERIOR PARTITION WALL TYPES

- PW1 — 190mm CONCRETE BLOCK
- PW2 — 240mm CONCRETE BLOCK
- PW3 — 140mm CONCRETE BLOCK
- PW4 — 90mm CONCRETE BLOCK– 100% SOLID
- PW5 — 16mm GB–AR 92mm METAL STUDS @ 400 O.C. WITH

DRAWINGS CREATED USING INFORMATION FROM EX. DRAWINGS PREPARED BY:
(1955 ORIGINAL) SHORE & MOFFAT ARCHITECTS. PROJECT NO. 230 DATED 1956/02/06
(1969 ADDITION #2) SHORE & MOFFAT AND PARTNERS. PROJECT NO. 230D DATED 1969/10/20

LEVEL 1 PARTIAL FRAMING PLAN 1 S3.00
SCALE: 1:100

ROOF FRAMING PARTIAL PLAN - LIBRARY AND 1-STEM 2 S3.00
SCALE: 1:100

Client
Halton District School Board
2050 Guelph Line
Burlington, Ontario

T.A. BLAKELOCK H.S. RENOVATION

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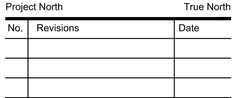
Key Plan N.T.S.



Project North True North

No.	Revisions	Date
1	ISSUED FOR BIDS / PERMIT	2023/03/13
No.	Issue	Date

Drawing Title:
LEVEL 0 RENOVATION PLANS

Scale:	AS NOTED	Date:	MARCH 2023
Drawn by:	JRD	Checked by:	EH
Job No.	2215A	Drawing No.	S3.00

DRAWINGS CREATED USING INFORMATION FROM EX. DRAWINGS PREPARED BY:
(1955 ORIGINAL) SHORE & MOFFAT ARCHITECTS. PROJECT NO. 230 DATED 1956/02/06
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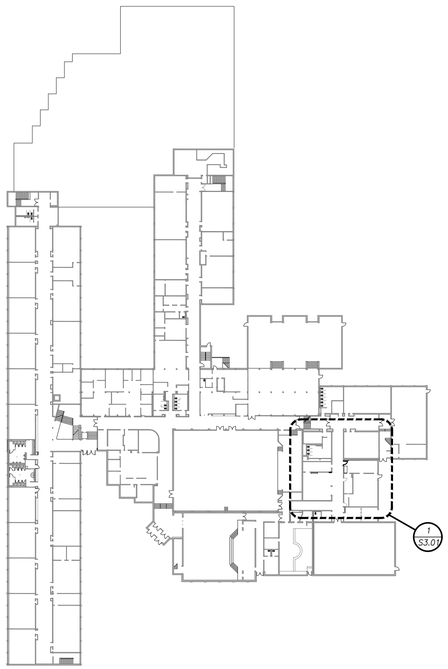
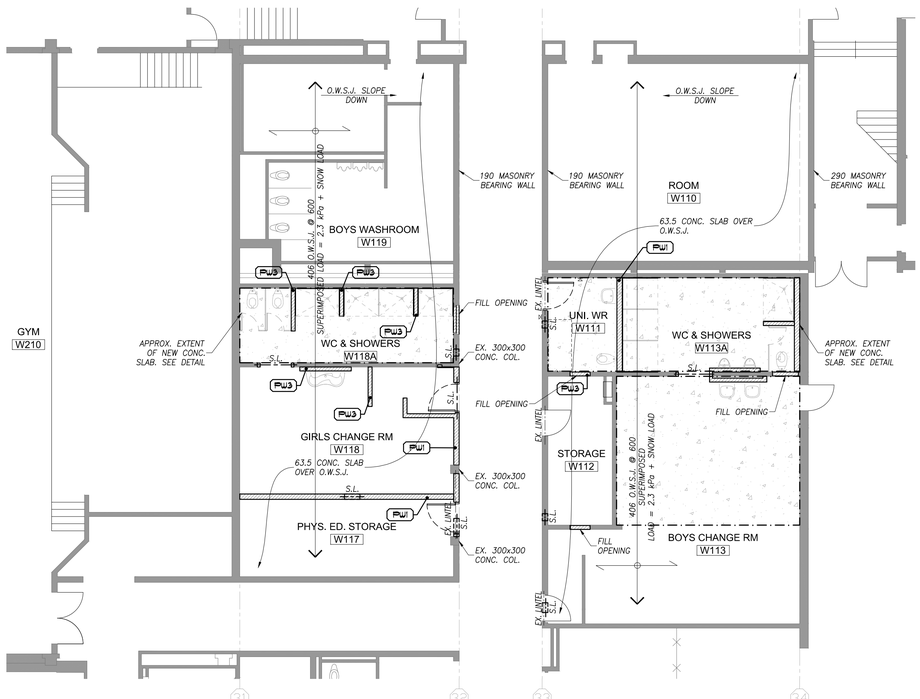


LEVEL 1 KEY - PLAN
SCALE: 1:750

INTERIOR PARTITION WALL TYPES

PW1	190mm CONCRETE BLOCK
PW2	240mm CONCRETE BLOCK
PW3	140mm CONCRETE BLOCK
PW4	90mm CONCRETE BLOCK– 100% SOLID
PW5	16mm GB–AR 92mm METAL STUDS @ 400 O.C. WITH



ROOF FRAMING PARTIAL PLAN - STORAGE ROOMS (1 S3.01)
SCALE: 1:100

Client
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Key Plan N.T.S.

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Drawing Title:
LEVEL 1 RENOVATION PLAN

Scale:	AS NOTED	Date:	MARCH 2023
Drawn by:	JRD	Checked by:	EH
Job No.	2215A	Drawing No.	S3.01